

APPENDIX B

GEOTECHNICAL ENGINEERING

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GENERAL

The proposed flood control plans include elements to enhance the channel conveyance (overflow swales) and levee elements. The overflow swales are referred to as the Chain of Wetlands, and the levee elements include the Lamar Street Levee and the Cadillac Heights Levee.

The Chain of Wetlands consists of an upper and lower swale separated at the IH-45 bridge. The upper swale alignment would have an approximate 400-foot average bottom width and would extend from the Cedar Creek confluence downstream to the oxbow at IH-45, a distance of approximately 1.5 miles. The upper swale would be aligned as far west of the Trinity River as possible to avoid adverse impacts to woodlands. The lower swale would have a 600-foot average bottom width between IH-45 and Loop 12, a distance of approximately 2.2 miles, and would be aligned through the Linfield Landfill and Sleepy Hollow Golf Course at the lower end.

The east, or Lamar Street, levee extension would connect the downstream end of the existing Dallas Floodway East Levee, at the east abutment of the old AT&SF RR bridge, with the existing Rochester Park Levee, at the east abutment of the Southern Pacific RR bridge. This levee extension would have an approximate average height of 20 feet and would be about 3 miles long. This extension would not require raising any portion of the existing Floodway East Levee. Approximately 1,000 feet of the Rochester Park Levee would be raised by an amount less than one foot. About 4,500 feet of the existing Rochester Park levee would be made unnecessary by the Lamar Street levee. Although the alignment of this levee extension would be adjacent to several commercial businesses, the majority of these businesses would not require relocation. The Proctor and Gamble storage facility and some smaller commercial structures at the downstream end of the Lamar Levee extension, near State Highway 310, would, however, require relocation.

The west, or Cadillac Heights, levee would begin upstream near Cedar Creek and extend downstream to the existing Central Wastewater Treatment Plant (CWWTP) Levee, would utilize a portion of the existing CWWTP Levee along the northwest corner at the plant entrance and then extend from the west side of the CWWTP Levee to high ground near the intersection of Kiest Boulevard and McGowan Avenue. The average height of this levee would be about 15 feet and the length from the Cedar Creek end to the downstream end near Kiest and McGowan would be approximately 2.3 miles. Construction of this levee may affect a number of commercial structures. Plate C-01, Sequence No. 2, presents a project location map.

GEOLOGIC DESCRIPTION

The area encompassed by the proposed Dallas Floodway Extension is underlain by limestone, marl (defined as halfway between limestone and shale), and shale representing the Austin Chalk Formation of Cretaceous age. More specifically, the project area is underlain by the Middle unit of the Austin Formation. The Middle unit is composed mainly of thinly bedded, laminated marl and shale, often indurated. The Austin Chalk Formation is, in turn, overlain by a thick mantle of alluvial water-bearing sediments which have been deposited by the Trinity River. The soft limestone and calcareous shales of the Austin Chalk outcrop can be found immediately beneath the overburden, beyond the floodplain on either side of the river. Seismic risk is insignificant at the project site.

The alluvial sediments encountered consisted variably of silty and sandy clays. Occasionally, significant lenses of water-bearing sands or gravelly sands were penetrated by exploratory borings. Extremely wet material with high humus content, was encountered near the surface in some areas.

SUBSURFACE INVESTIGATIONS

Subsurface investigations for the Dallas Floodway Extension consisted of a total of 138 borings, 115 completed in 1982, and 23 completed in 1997. The borings completed in 1997 are as follows: 400-410, 412-418, 427, 429, 430, 432 and 437. All other borings shown on the drawings were advanced in 1982. The 1982 borings were along the plan alignments investigated for the 1982 General Design Memorandum, consisting of levees along both sides of the Trinity River channel, as well as channelization of the Trinity River, Five Mile Creek and White Rock Creek. The total number of borings advanced in 1982 was actually greater than 115, but many of the borings were in areas outside the scope of the current plan (1998 General Reevaluation Report). In order to fully maximize drilling resources, subsurface data gained from the borings advanced in 1982 was used extensively in the current design. Additional subsurface investigations should be conducted in order to sample and test soils where information is needed to design a specific structural feature.

A boring location plan is presented on plates B-1 through B-3. The borings provided undisturbed samples from core barrels, Denison barrels, and Shelby tubes. Jar samples and bag samples were obtained from auger cuttings.

SUBSURFACE CONDITIONS

Subsurface soil profiles are presented on plates B-4 through B-14. The profile for the Lamar Levee is on plates B-4 through B-9, and the profile for the Cadillac Heights Levee is on plates B-10 and B-11. The profile for the downstream swale is on plates B-12 and B-13, and the profile for the upstream swale is on plates B-14 and B-15. The profile for the slurry trench is on plate B-16. Please note that many of the boring logs used to construct these profiles are offset from the centerline of the feature. Subsurface conditions encountered consist of the following:

Overburden

The overburden soils along the project alignment consist of undisturbed and disturbed alluvial and residual soils. The project site extends through highly developed urban lands which have undergone reshaping of terrain and quarrying of sands and gravels on a commercial scale. The overburden thickness as encountered in the borings varied considerably. It was less than ten feet thick in some places and more than twenty feet thick in others. Overburden thickness in the floodplain areas was generally in excess of twenty-five feet. For this project, these soils are segregated into three distinct soils groups:

Floodplain. This group is located within the distinct floodplain of the Trinity River and consists of clay of moderate to high plasticity. In general, disturbance of soils within this group has been minimal. Most of the features of the plan are located within the floodplain, and thus will encounter clays and sandy clays characteristic of floodplain deposits. Laboratory testing of these soils confirmed some extremely plastic soils, as shown by high liquid limits (99 in Boring 64), and high plasticity indices (59, also in Boring 64). Soil classifications consisted of CL, CH, SC, SM, with the occasional GC encountered.

Terrace. This group is located out of the floodplain, and forms the naturally occurring gentle bluff found on both sides of the Trinity River. This bluff is more prominent on the north side than the south side, where there isn't a well-defined boundary. The soils are predominantly clayey sands with occasional gravels. Some reshaping and disturbance has occurred. The only portion of the

project located near the terrace is the extreme southern end of the Cadillac Heights levee. This is to be expected, as terrace deposits are by definition the higher boundaries of the floodplain, and form the high ground that the project levees tie into. Laboratory testing of these soils revealed CL, CH, SC, SM and GC soils present. The proportion of coarse-grained soils encountered was higher than encountered in the floodplain. Plasticity of the clays ranged from low to high.

Urban. This group is located out of the floodplain along the south side of the Trinity River, and consists of man-made fills. The soils are intermixed clays, sandy clays, clayey sands, sands, and gravels, and, in the case of the Linfield Landfill, man-made construction debris and garbage such as tires, sheet metal, concrete blocks, ceramic tiles and steel I-beams. No laboratory testing was done on samples taken from the landfill.

Primary

Primary strata consist of weathered and unweathered limestone and shale.

GROUNDWATER

The water table was encountered in numerous borings with considerable variance in depth. Because of this variance, the subsurface profile should be viewed for the respective groundwater levels at each area. Alluvial deposits, as encountered during the borings and as described in the preceding paragraphs, located below the water table will necessitate shoring during excavation operations. Dewatering will be required during construction of the proposed project components. The excavations for the construction of some of the stoplog structures may encounter ground water and may require a dewatering system for construction in the dry. Care of water should also address control of storm runoff.

EXCAVATION EFFORT

The weathered shale and limestone will present no significant excavating problems. Furthermore, the overburden materials are of sufficient thickness over most of the project area that the primary will not be encountered during excavation. Standard excavation equipment should prove adequate for this project. More rigorous excavation equipment (e.g. heavy rippers) should not be needed.

LABORATORY TESTING

Conventional geotechnical testing of selected samples was conducted by the Southwestern Division Laboratory (CESWD) in Dallas, Texas and by the Materials Testing Center at Waterways Experiment Station. Tests for identification included tests for moisture content, grain size analyses, and Atterberg limits. Strength tests included triaxial (Q) tests, direct shear (S) tests, and unconfined compression tests. Testing results, as available in time for publication, are presented on the plates.

LEVEE MATERIAL

Material for construction of the levee embankments will come from required excavation of the overflow swales and, to a lesser extent, from required excavation of the stoplog structures and from the sumps. Subsurface explorations have encountered clays and sandy clays, which are suitable for levee construction. Some materials may require "drying back" prior to compaction. It is estimated that the volume of excavated material will exceed the levee fill requirements by approximately 600,000 cubic yards.

LEVEES

Embankments

The recommended levee section is presented on Plate C-03, Seq No. 4. The crest width of the levees will be 20 feet, and the side slopes of the levee embankments are recommended to be 1V:4H. This side slope configuration is required in order to prevent skin failures common on steeper embankments in the area. On the existing Dallas Floodway, surface failures on the 1V:3H side slopes are quite common, with approximately six occurring per year. The levees surrounding the Central Wastewater Treatment Plant (CWWTP) were originally constructed with slopes ranging from 1V:2.5H to 1V:3.3H. Numerous surface failures of the levees (approximately two per year), prompted the CWWTP to regrade their levee slopes to a 1V:4H slope during 1993-1994. The material used to regrade the CWWTP slopes was excavated from a nearby wetland mitigation area, and consisted mainly of CH soils, but also included CL soils. Since this regrading, there have been no slides on these levees. Preliminary stability analyses, using the Corps' UTEXAS3 program, showed a factor of safety against skin slides less than unity for a 1V:3H side slope, but indicated an adequate factor of safety for a 1V:4H side slope. More detailed slope-stability analyses will be conducted during the detailed design of this project. The UTEXAS 3 analysis used the following data:

Shear Strength	c = 0, phi = 18 deg
Unit weight	Moist: 105 pcf, sat'd: 114 pcf
Type of case analyzed	Steady seepage, no floodwater loading
Type of failure	non-circular
Procedure used	Spencer's

The maximum height of the Lamar Street levee will be 31 feet (at station 67+00), but will generally be less than 20 feet along the remainder of the levee alignment. The maximum height of the Cadillac Heights levee will be less than 26 feet, with an average height of less than 15 feet.

The levee embankments will be constructed of suitable clays (CL and CH) and suitable clayey sands (SC) from the required excavation. The embankment fill should be compacted to a minimum relative compaction of 95 percent standard Proctor density (ASTM 698) at, or slightly above, optimum moisture content.

Underseepage

It is recommended that an inspection trench be excavated along the levee alignments. The purpose of the trench is to disclose undetected utilities or other foundation problems and to reduce potential seepage problems. The inspection trench should be excavated to a depth of 5 feet, with a bottom width of 10 feet. Side slopes on the inspection trench will be 1V:1H. Underseepage is not foreseen to be a problem, due to the generally impervious nature of the alluvial riverbed sediments. However, in order to lengthen the seepage path, the inspection trench will be backfilled with impervious fill material. Alluvial riverbed sediments typically are lensatic in nature; that is, pockets of sandy, permeable material that would readily convey seepage are discontinuous, and can grade both horizontally and vertically into impermeable clay or clayey sand over short distances. Therefore, problem seepage areas are not expected to occur over large, continuous areas.

GEOTECHNICAL PARAMETERS FOR STRUCTURES

The geotechnical parameters for design of structures are as follow:

Floodplain (Clay as CL and CH)

MKT Railroad (Lamar Street Levee)
Martin Luther King, Jr. Boulevard

Above Normal Water Table

Unit weight, moist: 118 pcf (MC = 23%)
Unit weight, dry: 94 pcf

Allowable bearing capacity = 2500 psf

Shear strength (S): $c = 0$, $\phi = 18^\circ$
Sliding on concrete: $c = 0$, $\phi = 12^\circ$

At rest, $k_o = 0.5$
Active, $k_a = 0.5$
Passive, $k_p = 2.0$

Below Normal Water Table

Unit weight, moist: 105 pcf, MC = 20%
Unit weight, dry: 88 pcf
Unit weight, saturated: 114 pcf, MC = 30%

Shear strength (S): $c = 0$, $\phi = 18^\circ$
Sliding on concrete: $c = 0$, $\phi = 12^\circ$

At rest, $k_o = 0.8$
Active, $k_a = 0.8$
Passive, $k_p = 2.0$

Terrace (Clayey Sand as SC)

Southern Pacific Railroad
Central Expressway

Unit weight, moist: 128 pcf, MC = 13%
Unit weight, dry: 113 pcf
Unit weight, saturated: 135 pcf, MC = 18%

Allowable bearing capacity = 2500 psf

Shear strength (S): $c = 0$, $\phi = 28^\circ$
Sliding on concrete: $c = 0$, $\phi = 19^\circ$

At rest, $k_o = 0.4$
Active, $k_a = 0.5$
Passive, $k_p = 2.6$

Urban (Clayey Sand and Sandy Clay as SC and CL-CH)

Sargent Road (Cadillac Heights)

Unit weight, moist: 122 pcf, MC = 18%

Unit weight, dry: 108 pcf

Unit weight, saturated: 130 pcf, MC = 20%

Allowable bearing capacity = 2000 psf

Shear strength (S): c = 0, phi = 25°

Sliding on concrete: c = 0, phi = 17°

At rest, $k_a = 0.4$

Active, $k_a = 0.5$

Passive, $k_p = 2.4$

SLURRY TRENCH

The slurry trench should be constructed between the Linfield Landfill and the downstream swale in order to prevent water migration and thus possible contamination of flood waters. Additionally, the slurry trench will aid in the dewatering of the excavation and construction of the swale in the dry. The slurry trench will be constructed roughly in an L-shape in plan, and will be approximately twenty-five feet deep and two feet wide (minimum). The base of the slurry trench will be excavated into undisturbed clays, or primary strata (shale or limestone). See Plate C-03, Seq. No. 4 for a typical cross section. The fill material for the trench will be a cement-bentonite mixture, with a bentonite-water slurry used to hold the trench open during construction. Table B-1 presents a breakdown of estimated quantities for the slurry trench.

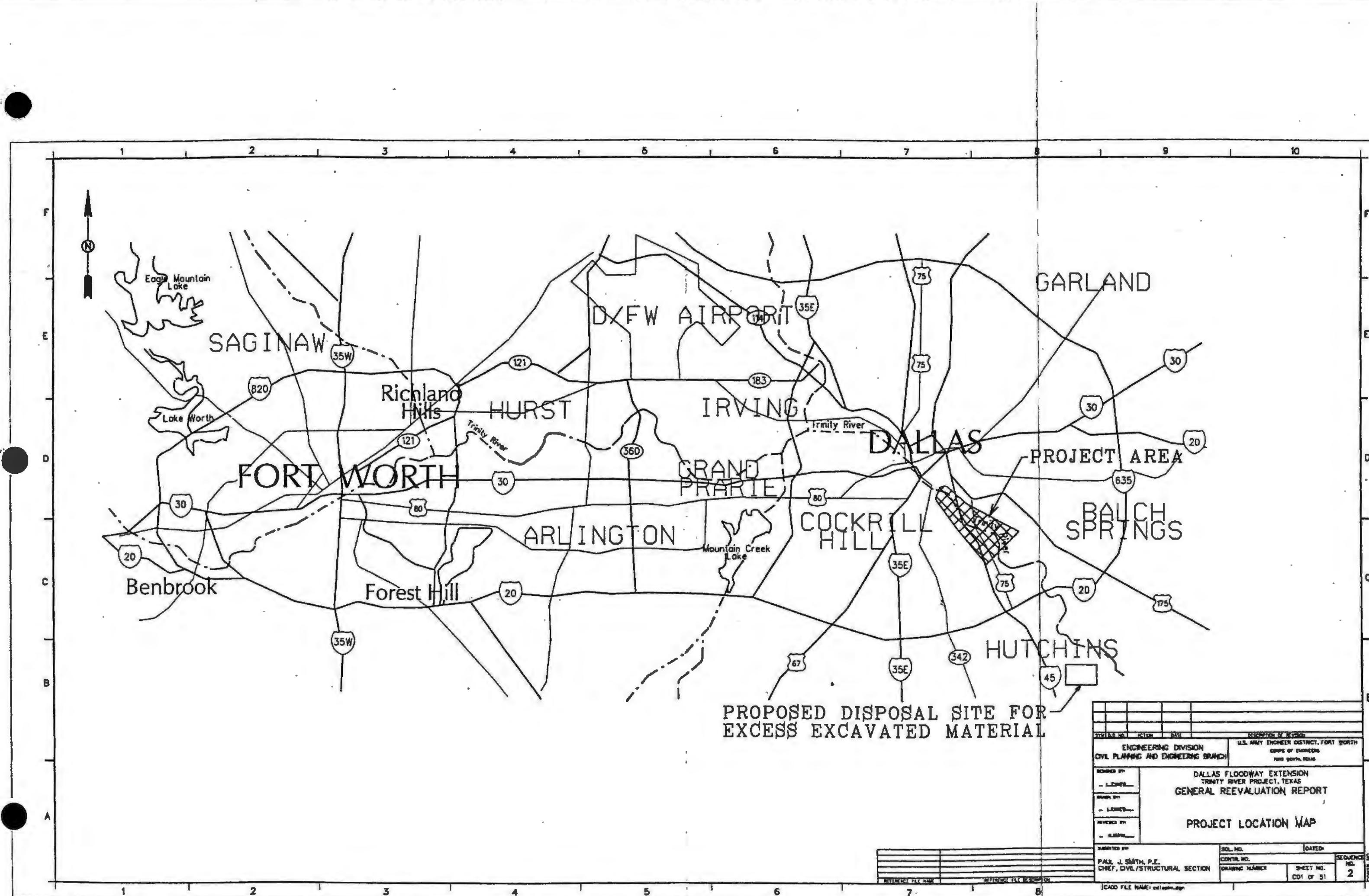
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Slurry Trench Quantities

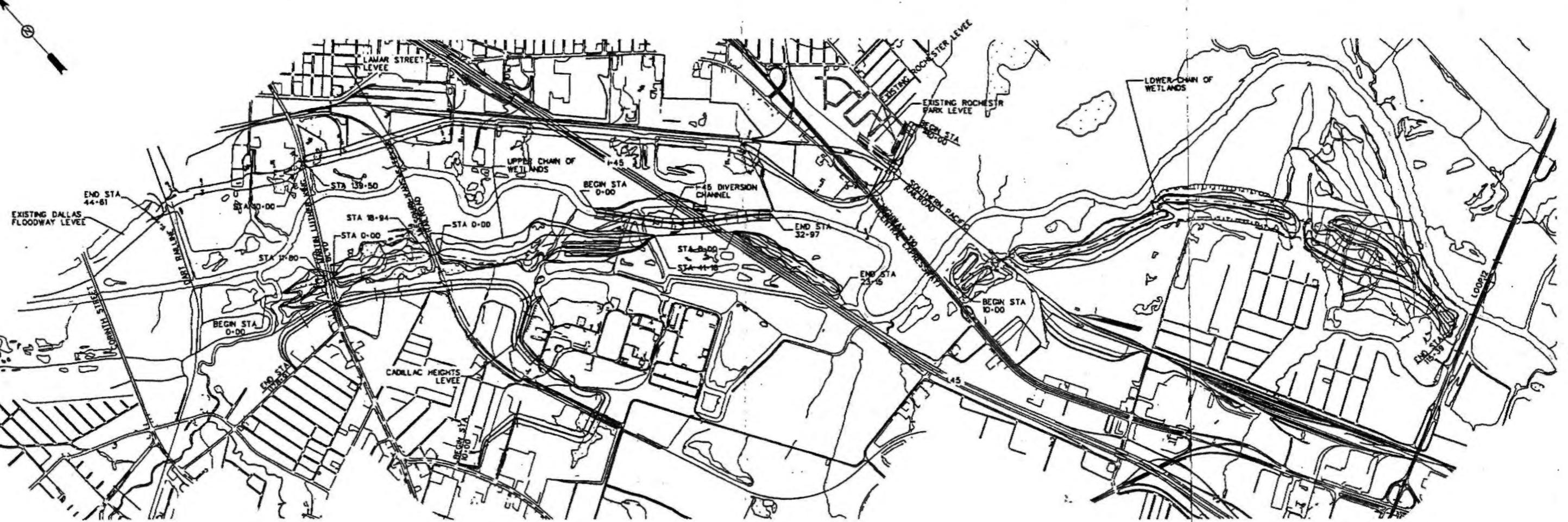
Item	Quantity
Excavation (Class I Non-Hazardous Material)	5,674 cubic yards
Slurry: Bentonite (10% by weight)	996 tons
Backfill: Cement* (6% by weight)	552 tons
Backfill: Soil **	5,334 cubic yards

*Portland cement to be used, in compliance with requirements of ASTM C150.

** Typical gradation requirements are as follows:

<u>Sieve Size</u>	<u>% Passing, by weight</u>
3"	100
1-1/2"	95-100
3/4"	80-100
No. 4	55-100
No. 10	40-80
No. 40	18-45
No. 200	10-25





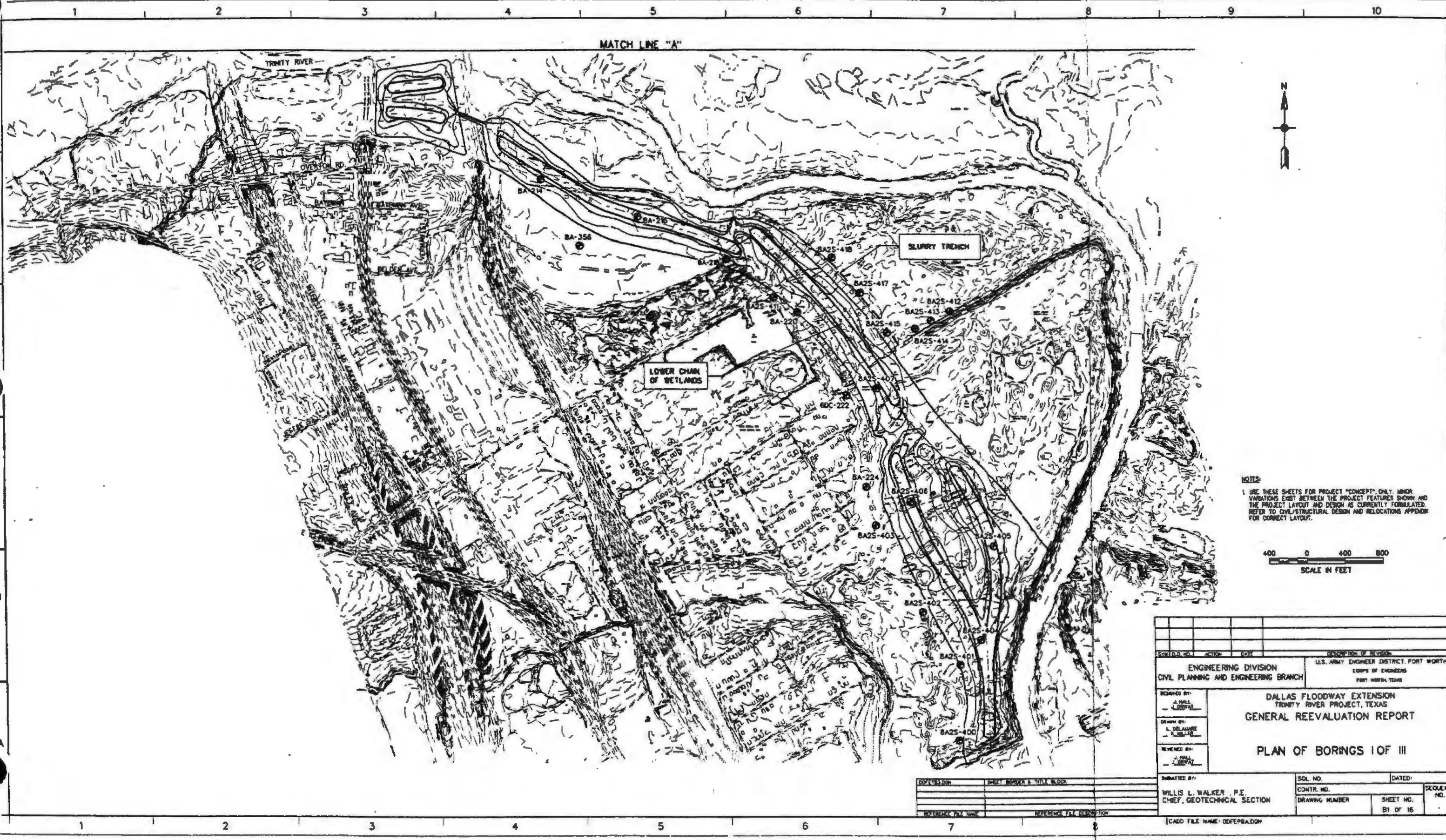
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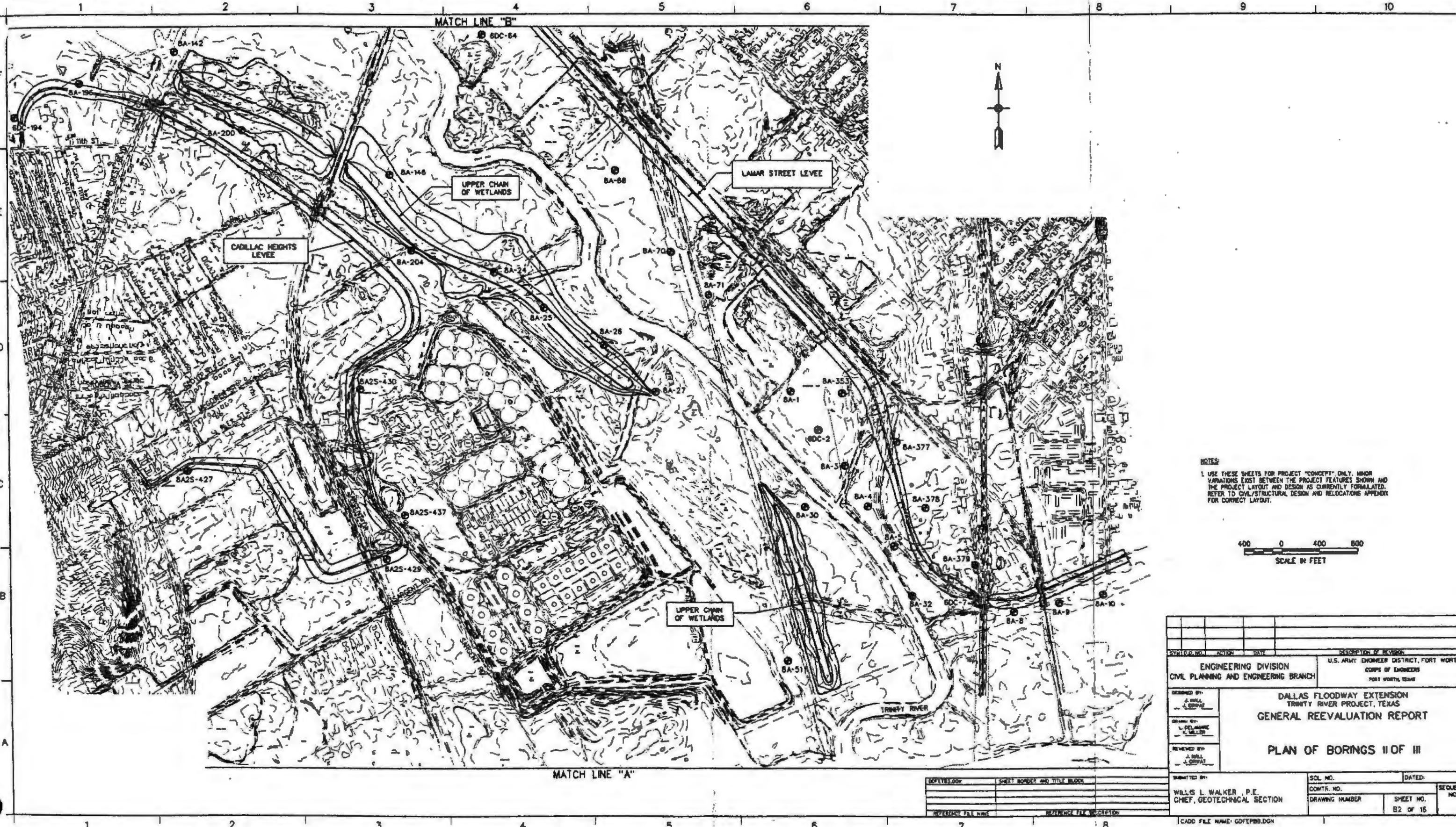
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SCENE AT EIFFEL





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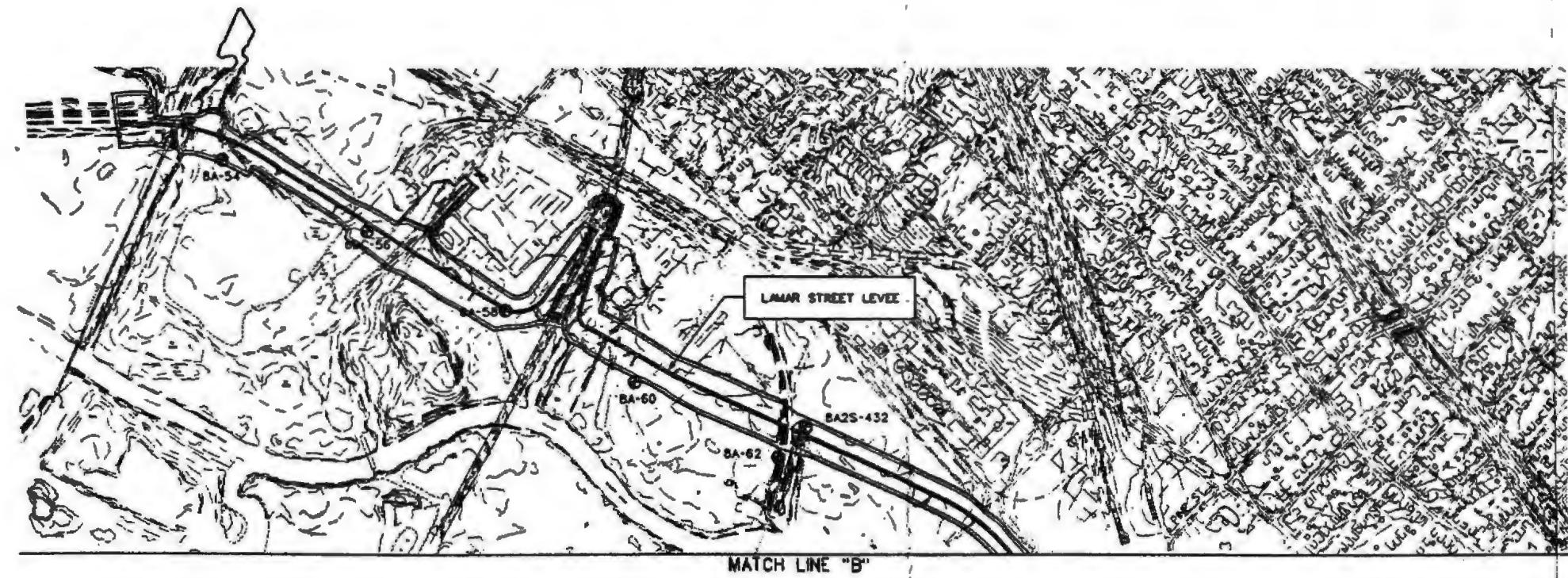
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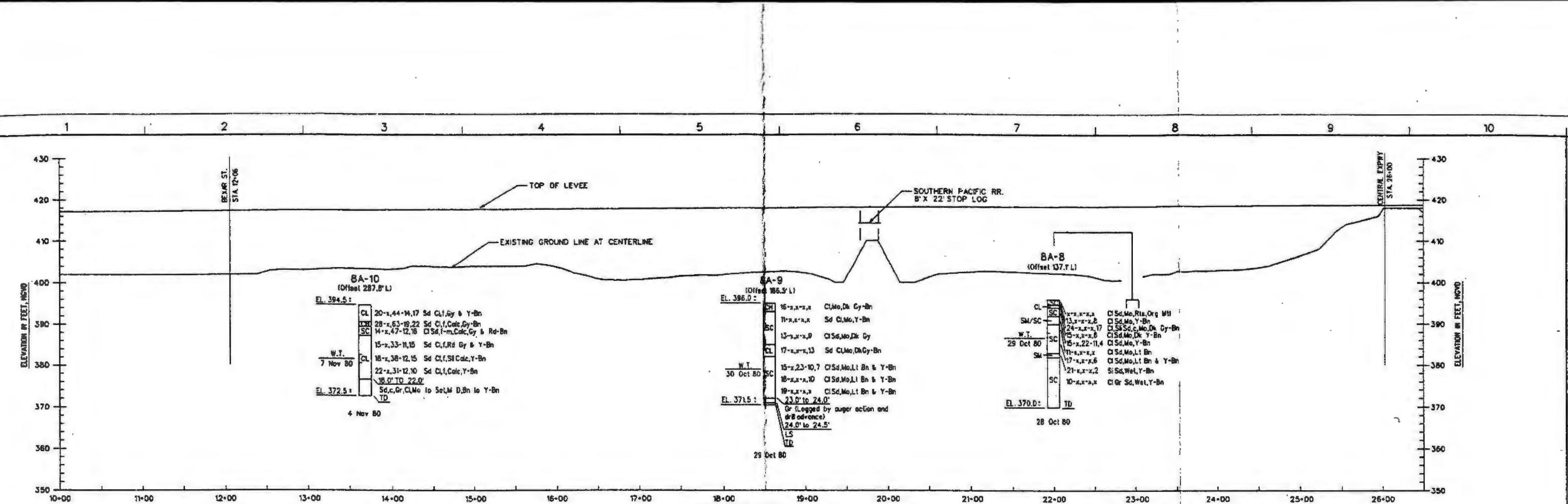


NOTES:
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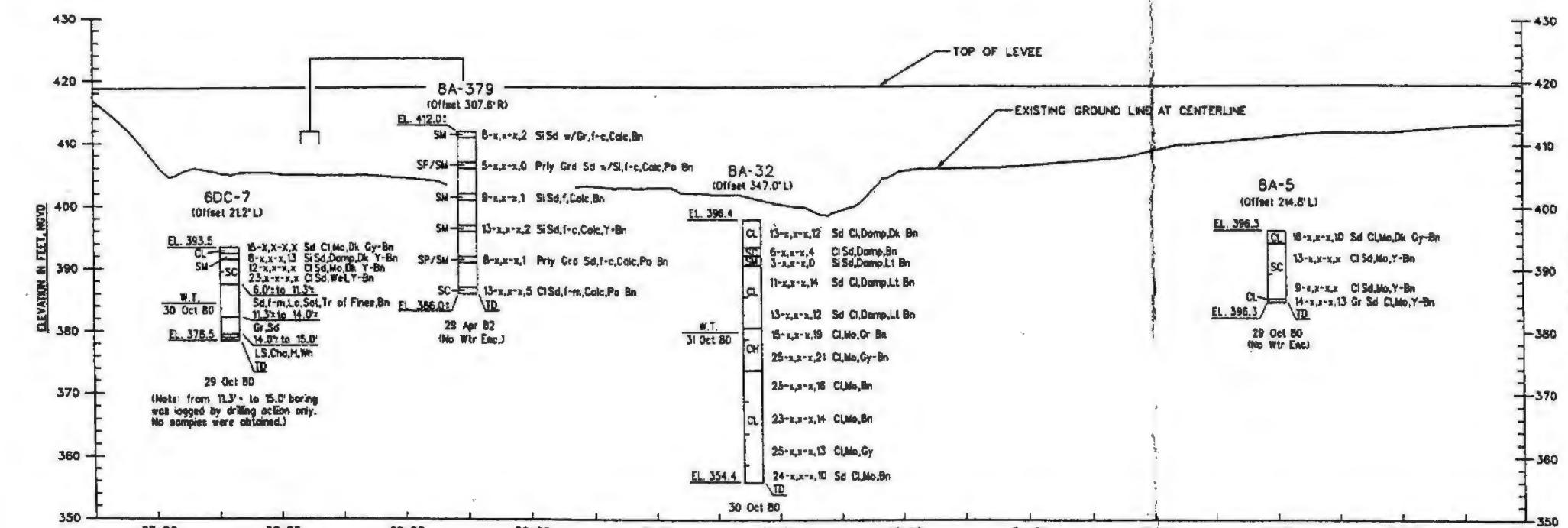
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REMOVED BY:	J. HALL — L. DELMAR —	REDRAWN BY:	DALLAS FLOODWAY EXTENSION TRINITY RIVER PROJECT, TEXAS GENERAL REEVALUATION REPORT
REVIEWED BY:	J. HALL — L. DELMAR —	PLAN OF BORINGS - II OF III	
COPIED FROM:		SHEET NUMBER & TITLE BLOCK	SOL. NO. DATED:
		WILLIS L. WALKER, P.E. CHIEF, GEOTECHNICAL SECTION	CONTR. NO.
REFERENCE FILE NAME:		REFERENCE FILE DESCRIPTION	DRAWING NUMBER
			SEQUENCE NO.
			B3 OF 16
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SUBSURFACE PROFILE ALONG LAMAR STREET LEVEE
(STATION 10-00 TO 26-50)



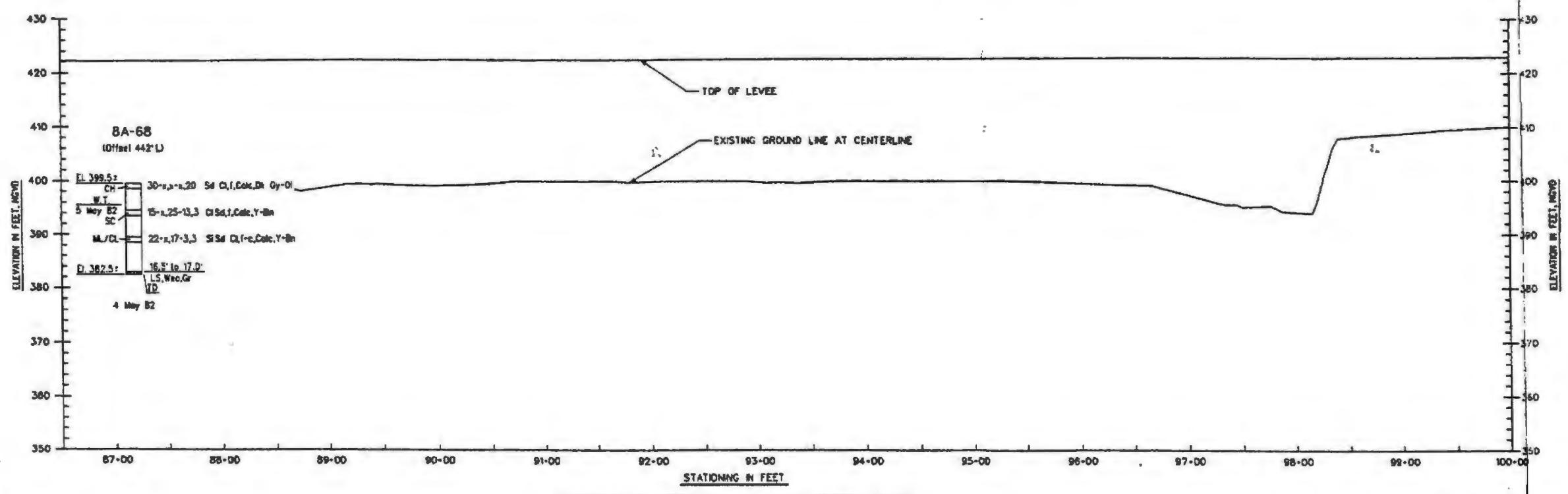
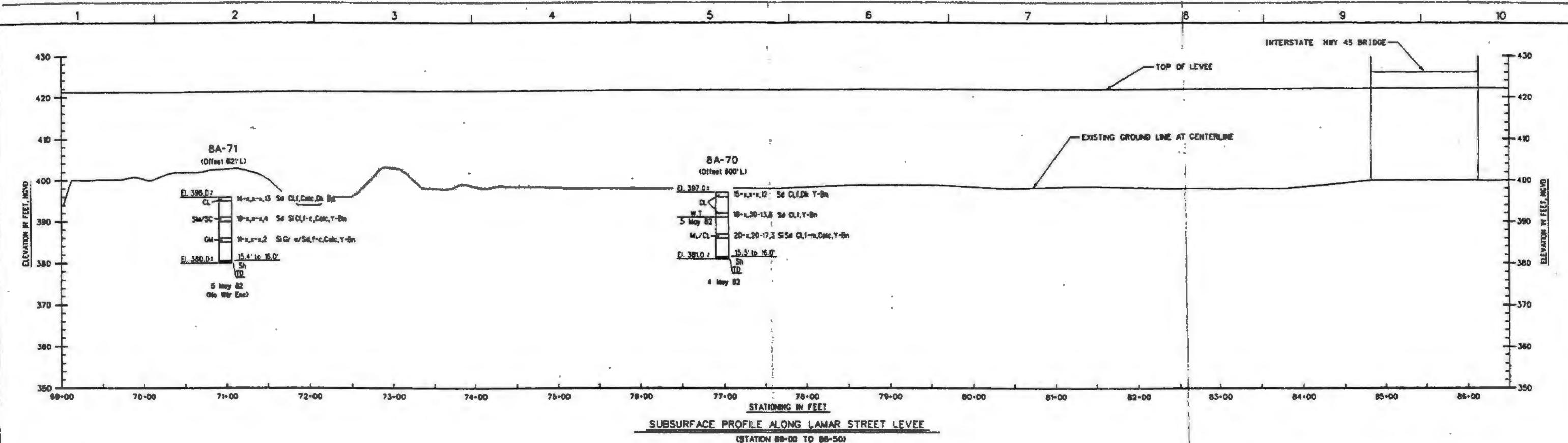
SUBSURFACE PROFILE - LAMAR STREET LEVEE
(STATION 26-50 THRU 38-00)

- NOTES:**
1. FOR BORING LOCATIONS, SEE DRAWING NO. B1 THROUGH B3.
 2. FOR LEGEND AND GENERAL NOTES, SEE DRAWING B9.
 3. USE THESE SHEETS FOR PROJECT "CONCEPT", ONLY. MINOR VARIATIONS EXIST BETWEEN THE PROJECT FEATURES SHOWN AND THE PROJECT LAYOUT AND DESIGN AS CURRENTLY FORMULATED. REFER TO CIVIL/STRUCTURAL DESIGN AND RELOCATIONS APPENDIX FOR CORRECT LAYOUT.

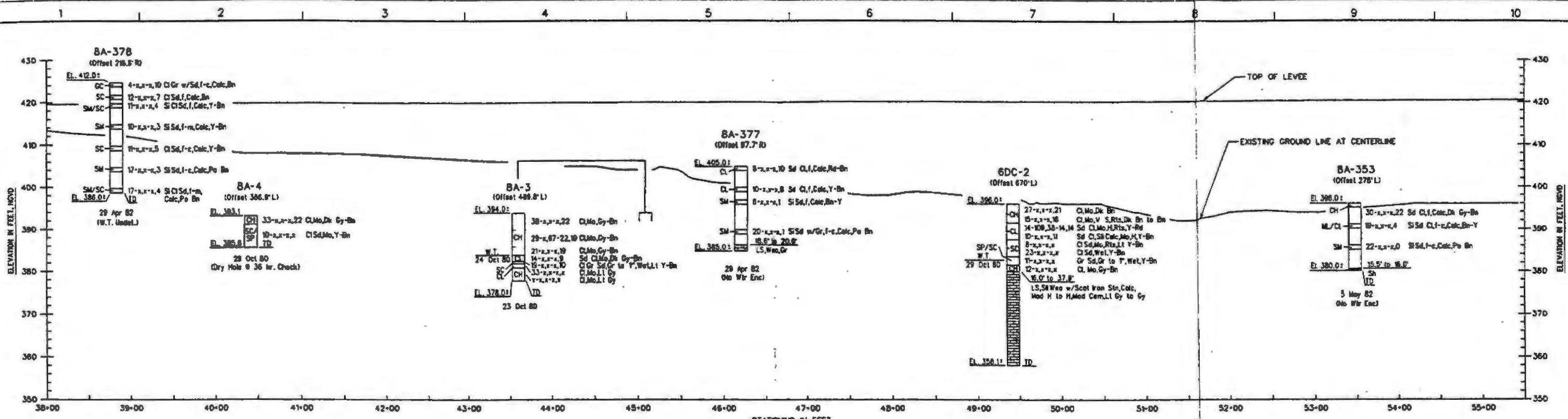
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VERTICAL SCALE IN FEET

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HORIZONTAL SCALE IN FEET

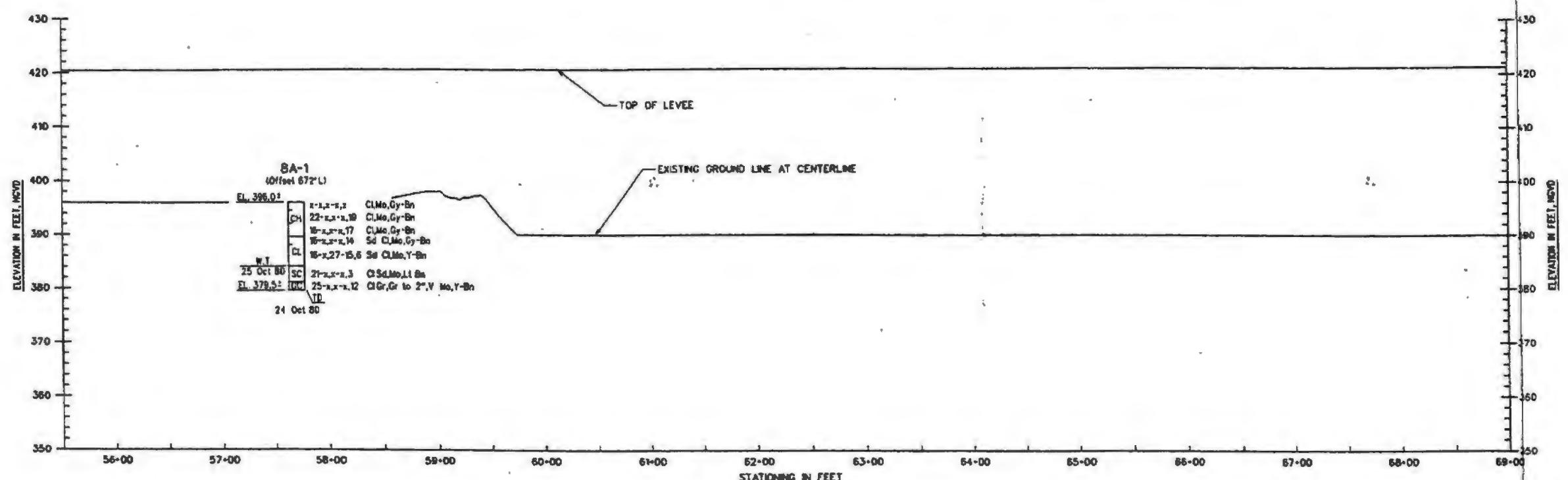
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K. MILLER		U.S. ARMY ENGINEER DISTRICT, FORT WORTH	
DRAWN BY:		CIVIL PLANNING AND ENGINEERING BRANCH	
K. MILLER		CORPS OF ENGINEERS	
REVIEWED BY:		FORT WORTH, TEXAS	
J. HALL		DALLAS FLOODWAY EXTENSION	
A. DIBBLEY		TRINITY RIVER PROJECT, TEXAS	
		GENERAL REEVALUATION REPORT	
		LAMAR STREET LEVEE	
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		(STATION 10-00 TO 38-00)	
SUBMITTED BY:		SOL. NO.	
WILLIS L. WALKER, P.E.		DATED:	
CHIEF, GEOTECHNICAL SECTION			
CONTR. NO.		SEQUENCE NO.	
DRAWING NUMBER		SHEET NO.	
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DEIGNED BY:	DALLAS FLOODWAY EXTENSION TRINITY RIVER PROJECT, TEXAS			
DRAWN BY:	GENERAL REEVALUATION REPORT			
REVIEWED BY:				
APPROVED BY:				
SUBMITTED BY:		SOL. NO.		DATED
WILLIS L. WALKER, P.E. CHIEF, GEOTECHNICAL SECTION		CONTR. NO.		
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				16 of 16
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SUBSURFACE PROFILE ALONG LAMAR STREET LEVEE
(STATION 38-00 TO 55-50)



SUBSURFACE PROFILE ALONG LAMAR STREET LEVEE
(STATION 55-50 TO 58-00)

FOR BORING LOCATIONS, SEE DRAWING NO. B1 THROUGH B3.
FOR LEGEND AND GENERAL NOTES, SEE DRAWING B9.
USE THESE SHEETS FOR PROJECT "CONCEPT", ONLY. MINOR
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FOR CORRECT LAYOUT.



VERTICAL SCALE IN FEET



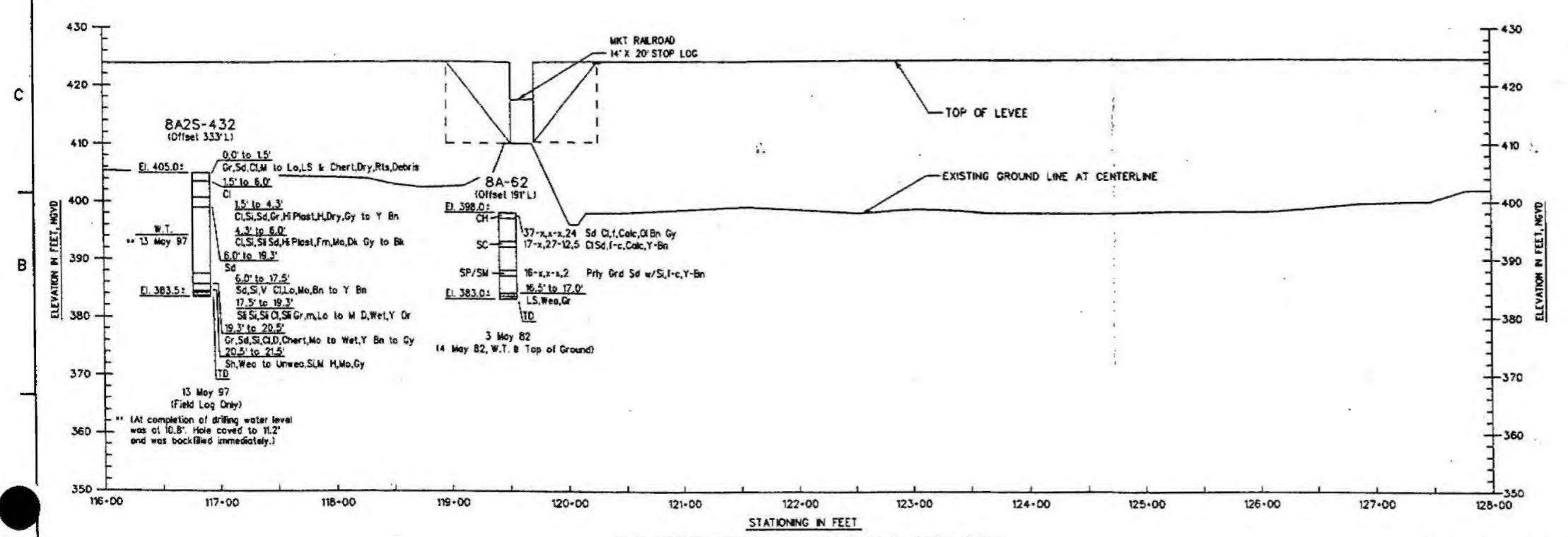
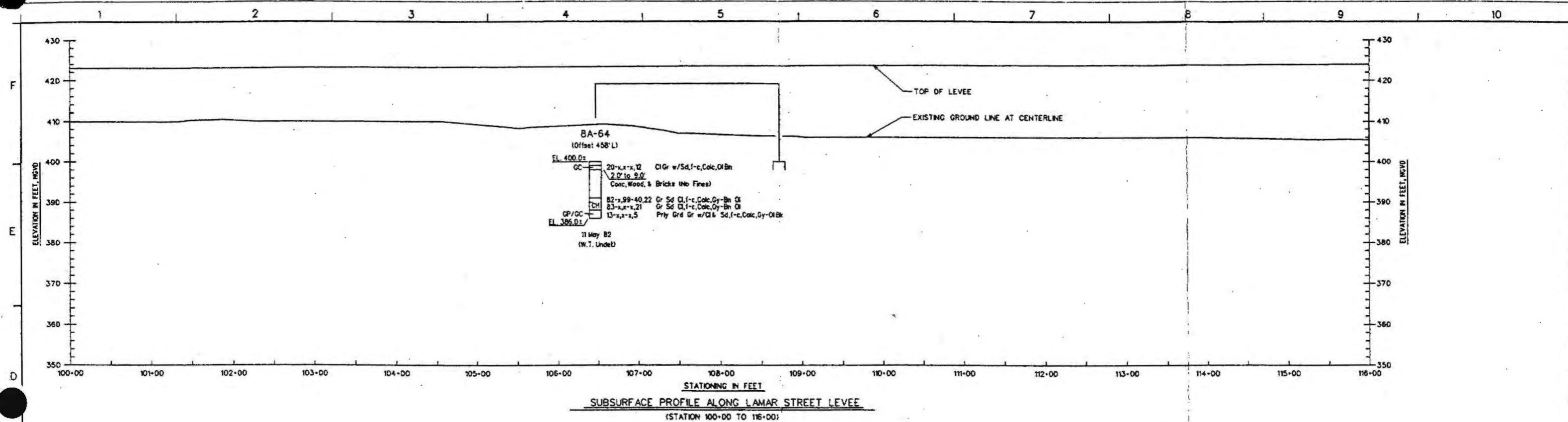
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ENGINEERING DIVISION					
DECODED BY: <u>J. WALKER</u>	DALLAS FLOODWAY EXTENSION TRINITY RIVER PROJECT, TEXAS				
DRAWN BY: <u>J. WALKER</u>	GENERAL REEVALUATION REPORT				
RECHECKED BY: <u>J. WILLIAMS</u> <u>J. BROWN</u>	LAMAR STREET LEVEE SUBSURFACE PROFILE (STATION 38-00 TO 69-00)				
SUBMITTED BY: WILLIS L. WALKER, P.E. CHIEF, GEOTECHNICAL SECTION		SOL. NO.	DATED:	SEQUENCE NO.	
		CONTR. NO.		SHEET NO.	
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CADD FILE NAME: GENE (SPD.GDN)					

DATE	DESCRIPTION OF REVIEW
S DIVISION	U.S. ARMY ENGINEER DISTRICT, FORT WORTH CORPS OF ENGINEERS

LAMAR STREET LEVEE
SUBSURFACE PROFILE
(STATION 38.00 TO 69.00)

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CONTR. NO.	
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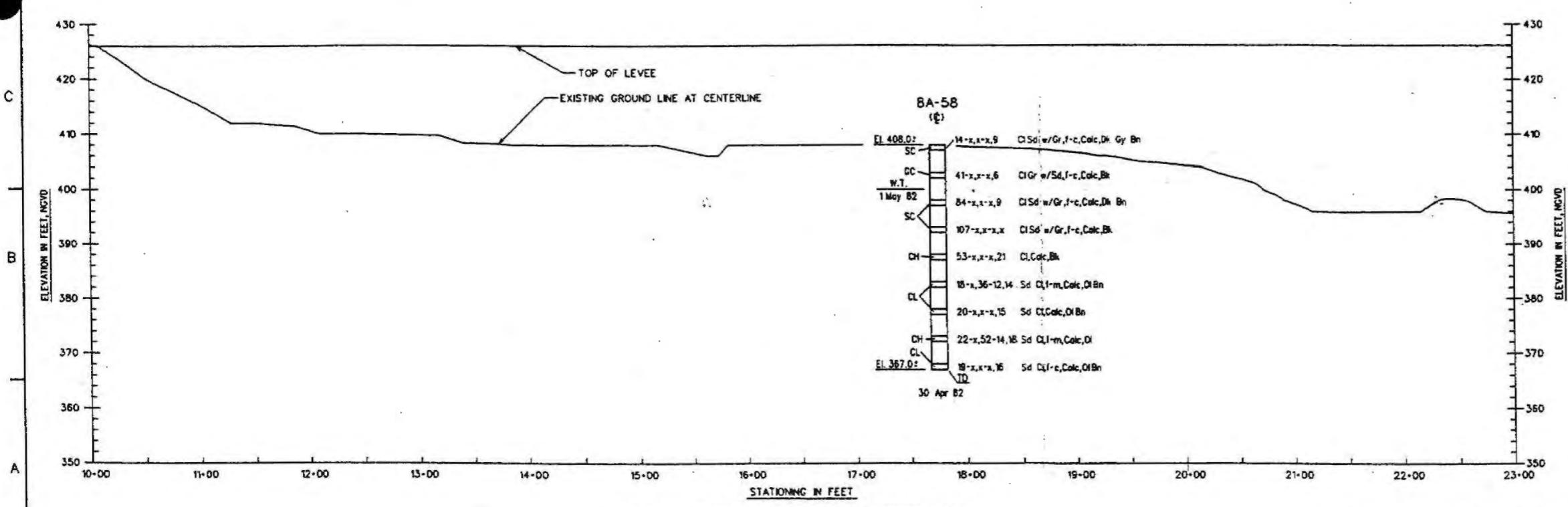
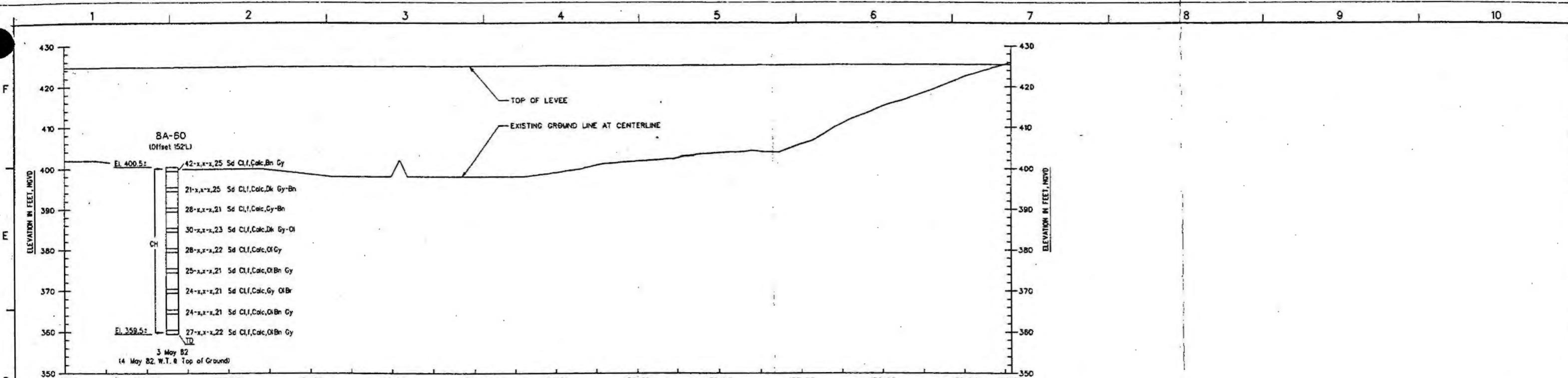


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DESIGNED BY:			DALLAS FLOODWAY EXTENSION TRINITY RIVER PROJECT, TEXAS
DRAWN BY:			GENERAL REEVALUATION REPORT
REVIEWED BY:			LAMAR STREET LEVEE SUBSURFACE PROFILE (STATION 100-00 TO 128-00)
SUBMITTED BY:			SOL. NO. DATED:
WILLIS L. WALKER, P.E. CHIEF, GEOTECHNICAL SECTION			CONTR. NO. SEQUENCE NO.
DRAWING NUMBER			SHEET NO.
B7 OF 16			SEQUENCE NO.

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- NOTES:
1. FOR BORING LOCATIONS, SEE DRAWING NO. B1 THROUGH B3.
 2. FOR LEGEND AND GENERAL NOTES, SEE DRAWING B9.
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VERTICAL SCALE IN FEET

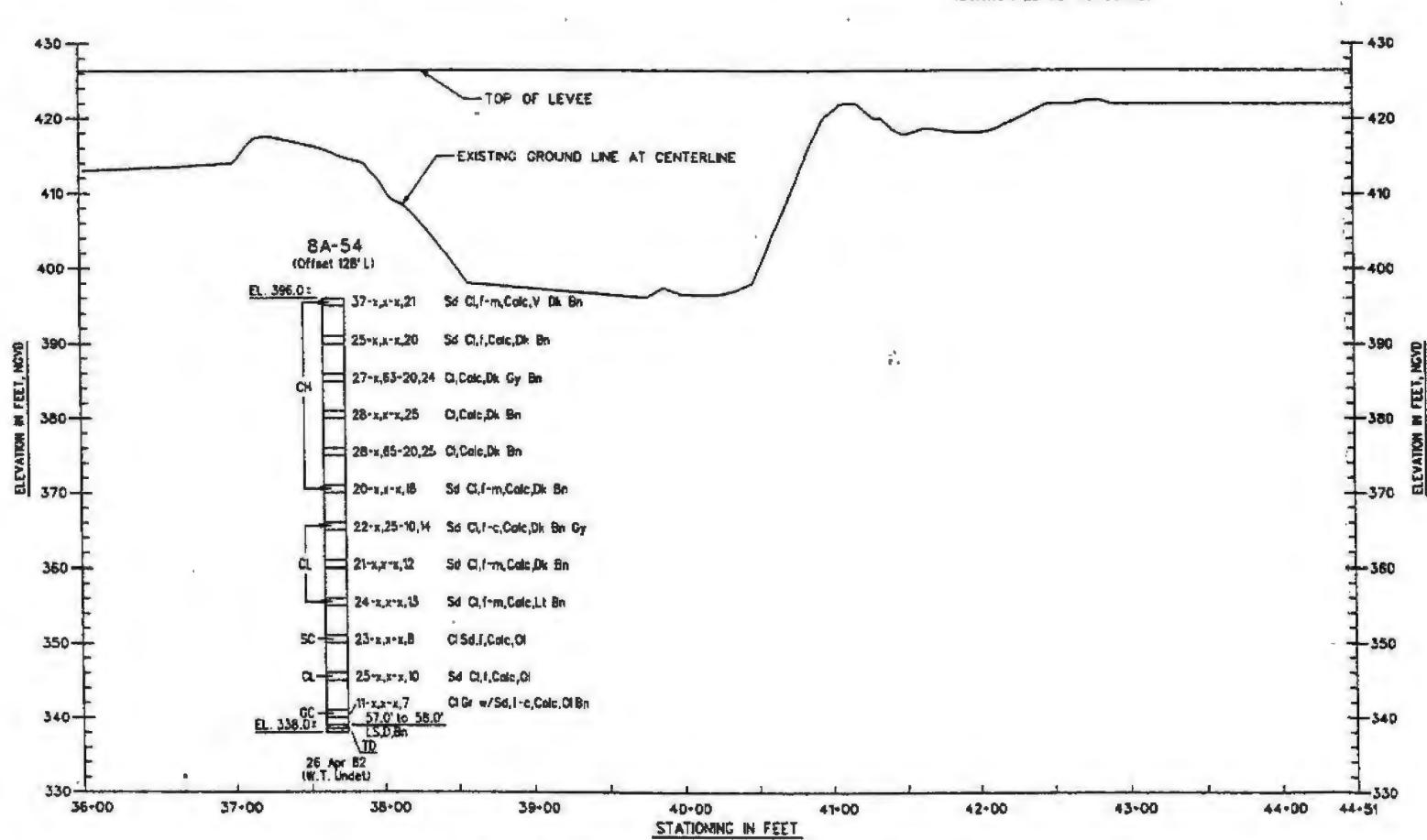
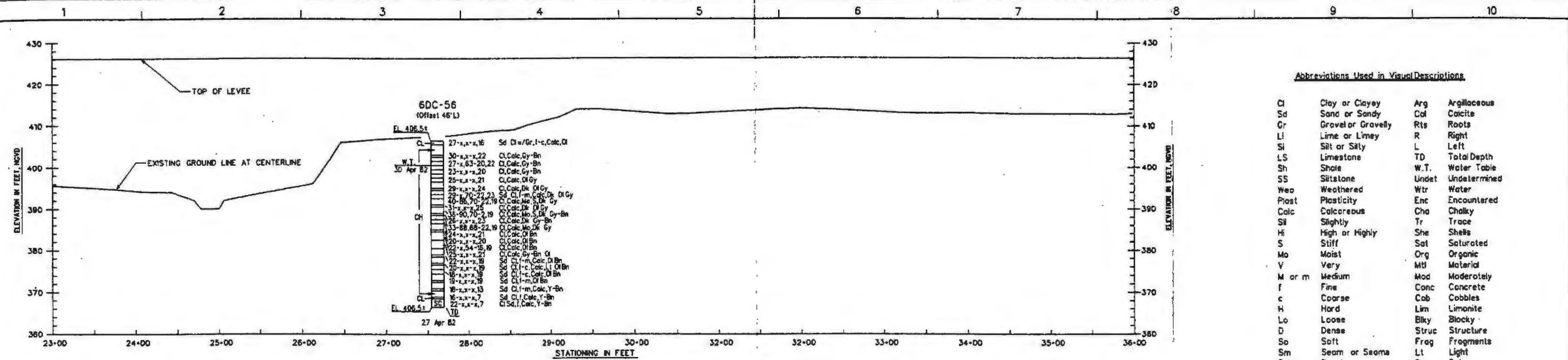
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HORIZONTAL SCALE IN FEET

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DEIGNED BY: F. MILLER	DALLAS FLOODWAY EXTENSION TRINITY RIVER PROJECT, TEXAS		
DRAWN BY: F. MILLER	GENERAL REEVALUATION REPORT		
REVIEWED BY: J. HALL J. ORRAT	LAMAR STREET LEVEE SUBSURFACE PROFILE (STATION 128.00 TO 139.58) (STATION 10.00 TO 23.00)		

SUBMITTED BY: WILLIS L. WALKER, P.E. CHIEF, GEOTECHNICAL SECTION	SOL. NO. CONTR. NO.	DATED: SEQUENCE NO.
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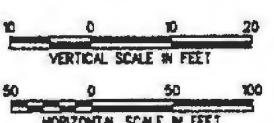
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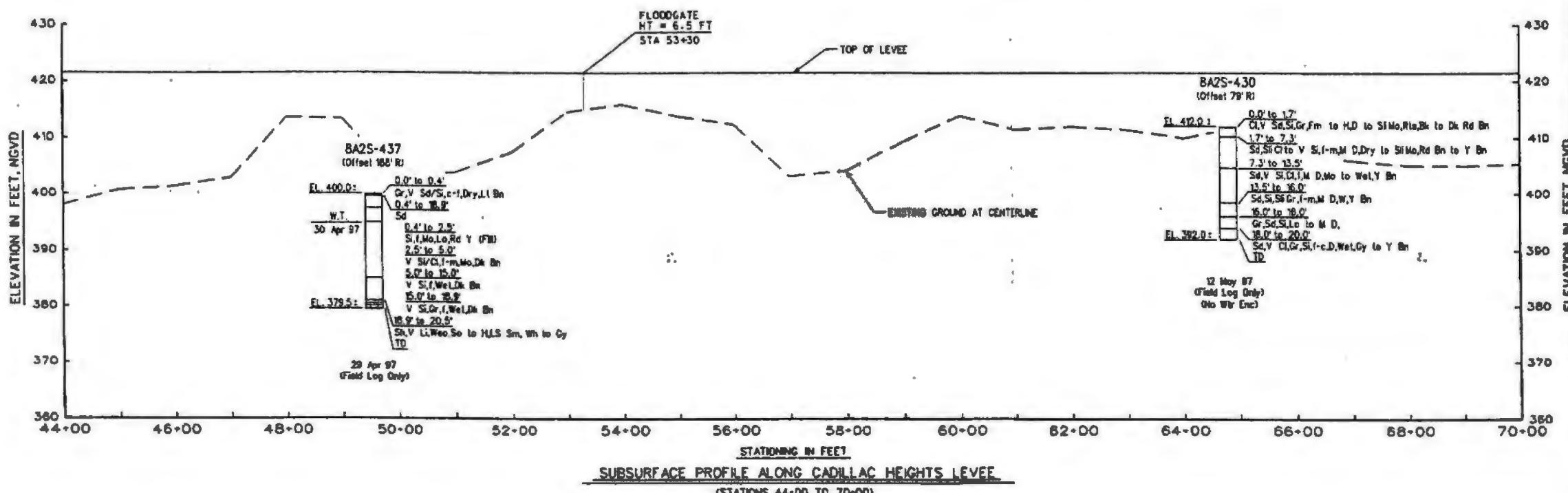
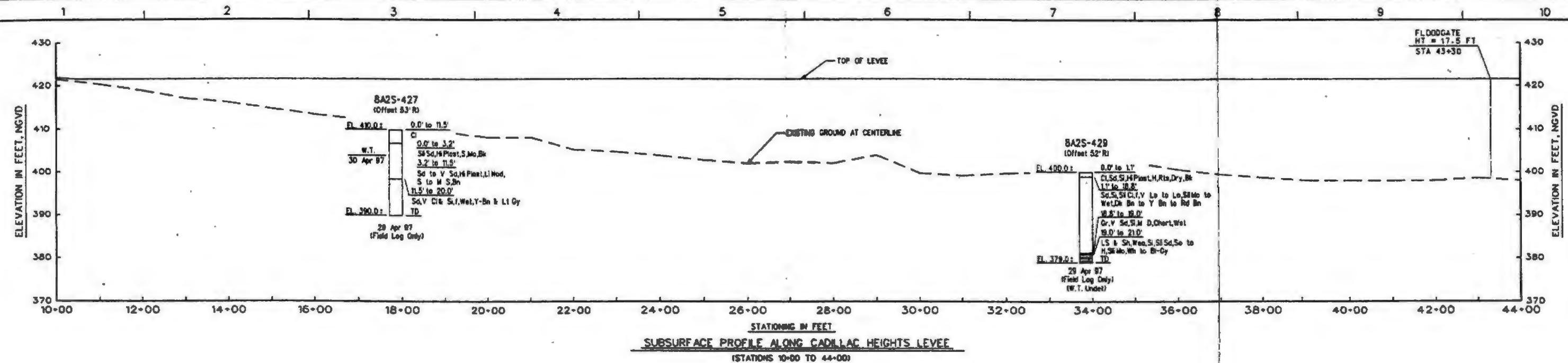
Sh	Shale
LS	Limestone

See ASTM D 2487 for complete details on the Unified Soils Classification System.



SYNOPSIS NO.	ACTION	DATE	DESCRIPTION OF REVISION
ENGINEERING DIVISION CIVIL PLANNING AND ENGINEERING BRANCH		U.S. ARMY ENGINEER DISTRICT, FORT WORTH COMPS OF ENGINEERS FORT WORTH, TEXAS	
DESIGNED BY: K. MILLER	DALLAS FLOODWAY EXTENSION TRINITY RIVER PROJECT, TEXAS		
DRAWN BY: K. MILLER	GENERAL REEVALUATION REPORT		
REVIEWED BY: J. PULL J. DRINKAT	LAMAR STREET LEVEE SUBSURFACE PROFILE (STATION 23-00 TO 44-51)		
SUBMITTED BY: WILLIS L. WALKER, P.E. CHIEF, GEOTECHNICAL SECTION		SOL. NO. CONTR. NO. DRAWING NUMBER SHEET NO. B9 OF 16	DATED: SEQUENCE NO.

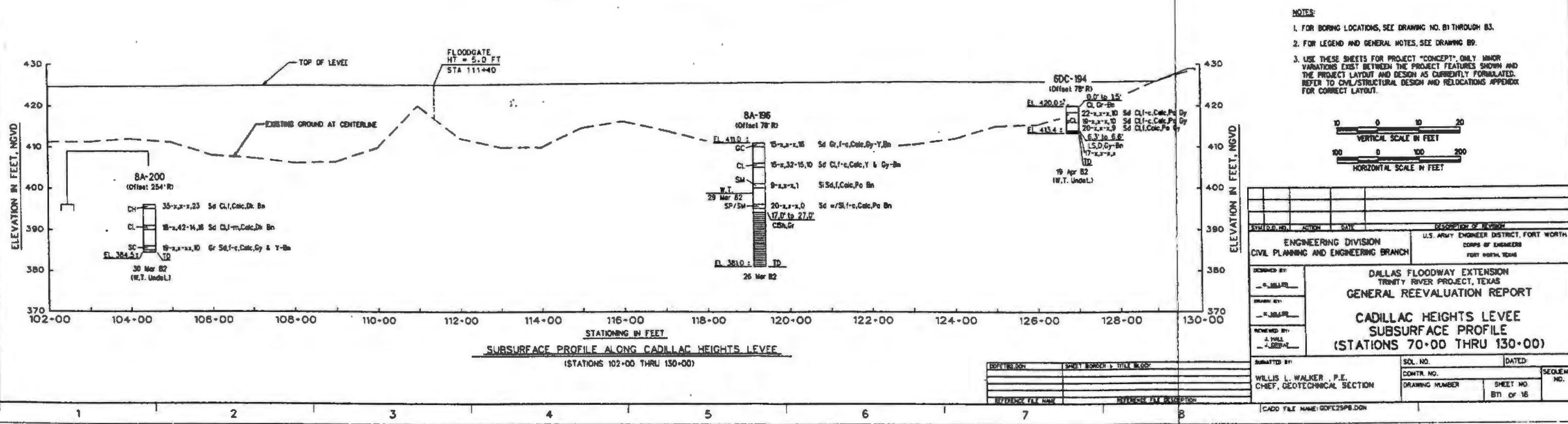
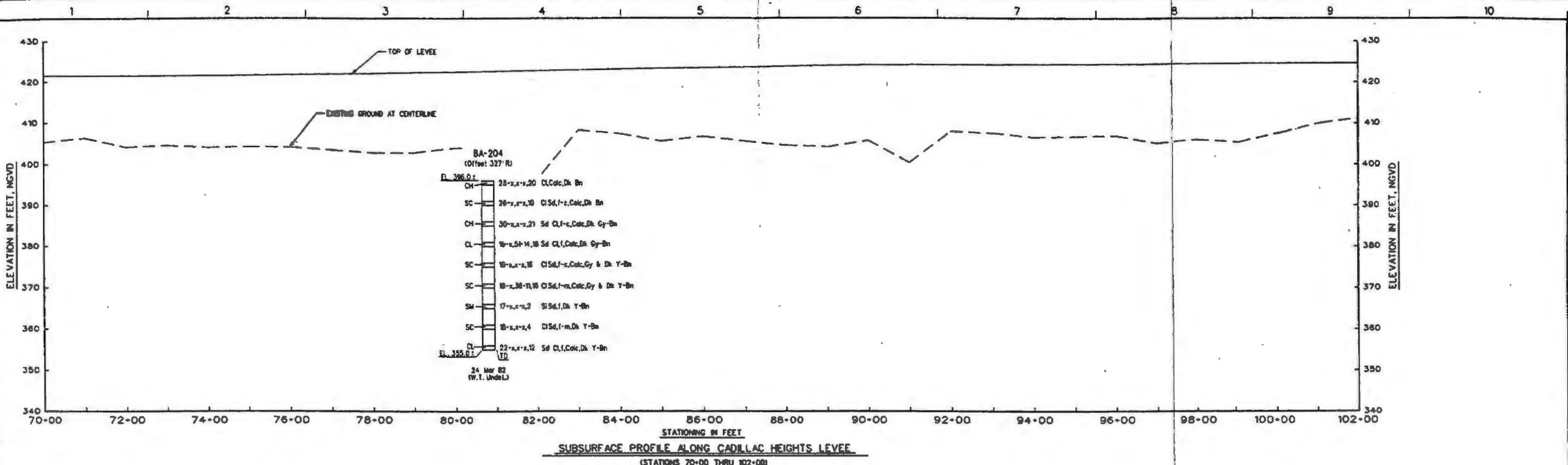
IACD FILE NAME: GDFE1SP.DGN

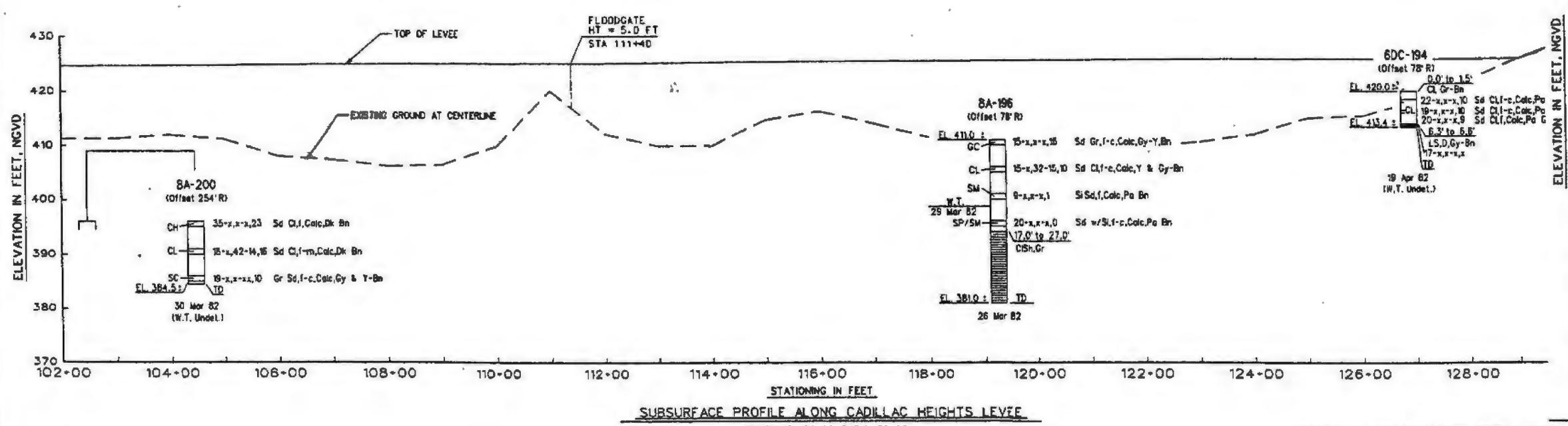
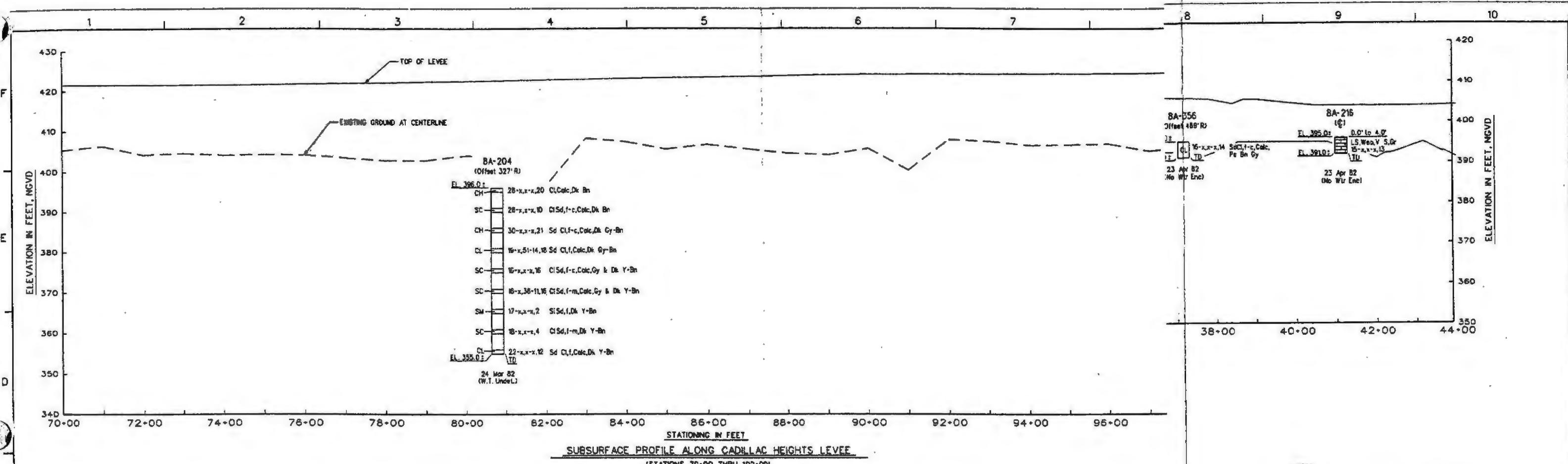


NOTES:

1. FOR BORING LOCATIONS, SEE DRAWING NO. B1 THROUGH B3.
2. FOR LEGEND AND GENERAL NOTES, SEE DRAWING B9.
3. USE THESE SHEETS FOR PROJECT "CONCEPT", ONLY. MINOR VARIATIONS EXIST BETWEEN THE PROJECT FEATURES SHOWN AND THE PROJECT LAYOUT AND DESIGN AS CURRENTLY FORMULATED. REFER TO CIVIL/STRUCTURAL DESIGN AND RELOCATIONS APPENDIX FOR CORRECT LAYOUT.

			DESCRIPTION OF REEVALUATION		
STAN. ID. NO.	ACTION	DATE	U.S. ARMY ENGINEER DISTRICT, FORT WORTH CORPS OF ENGINEERS FORT WORTH, TEXAS		
ENGINEERING DIVISION CIVIL PLANNING AND ENGINEERING BRANCH			<p style="text-align: center;">DALLAS FLOODWAY EXTENSION TRINITY RIVER PROJECT, TEXAS</p> <p style="text-align: center;">GENERAL REEVALUATION REPORT</p> <p style="text-align: center;">CADILLAC HEIGHTS LEVEE SUBSURFACE PROFILE (STATIONS 10+00 THRU 70+00)</p>		
DESIGNED BY: <u>K. WILLIS</u> DRAWN BY: <u>K. WILLIS</u> REVIEWED BY: <u>J. PELL</u> <u>K. WILLIS</u>		SUBMITTED BY: <u>WILLIS L. WALKER, P.E.</u> <u>CHEF, GEOTECHNICAL SECTION</u>		SOL. NO.	DATED:
		CONTR. NO.		SEQUENCE NO.	
		DRAWING NUMBER	SHEET NO.	B10 OF 16	

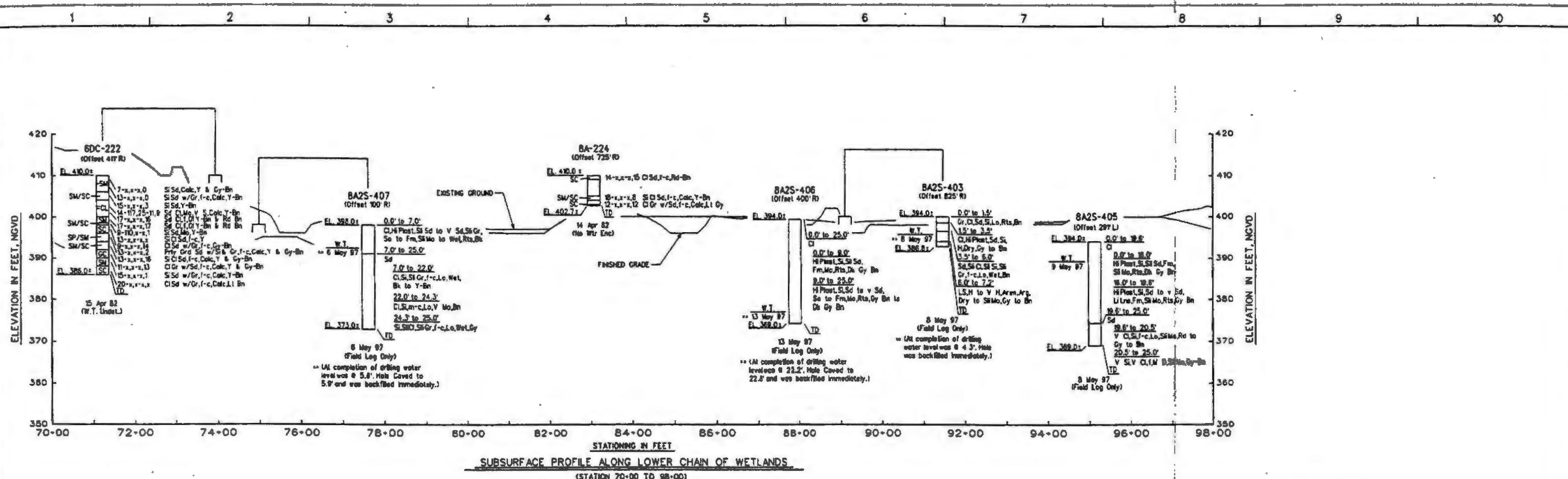




STN.D.D. NO.	ACTION	DATE	DESCRIPTION OF REVISION
			U.S. ARMY ENGINEER DISTRICT, FORT WORTH
			CORPS OF ENGINEERS
			FORT WORTH, TEXAS
REDESIGNED BY:	K. MILLER		DALLAS FLOODWAY EXTENSION
DRAINED BY:	K. MILLER		TRINITY RIVER PROJECT, TEXAS
REVIEWED BY:	J. HALL L. WALKER		GENERAL REEVALUATION REPORT
			LOWER CHAIN OF WETLANDS
			SUBSURFACE PROFILE
			(STATIONS 10-00 THRU 70-00)
SOL. NO.	DATED:		
CONTR. NO.			SEQUENCE NO.
DRAWING NUMBER			SHEET NO.
			B12 OF 16

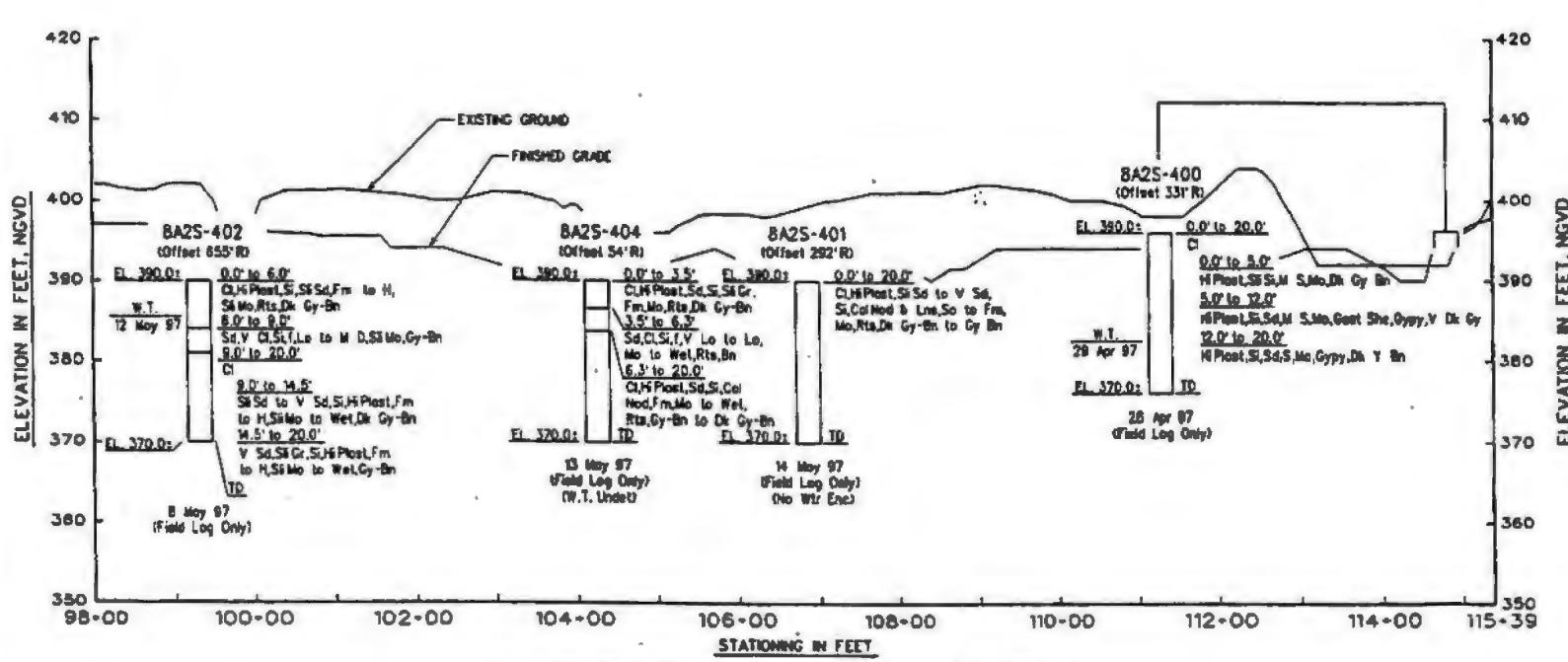
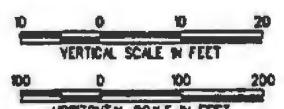
WILLIS L. WALKER, P.E.
CHIEF, GEOTECHNICAL SECTION

CADD FILE NAME: GOFSSP.DON



- NOTES:
1. FOR BORING LOCATIONS, SEE DRAWING NO. B1 THROUGH B3.
 2. FOR LEGEND AND GENERAL NOTES, SEE DRAWING B0.

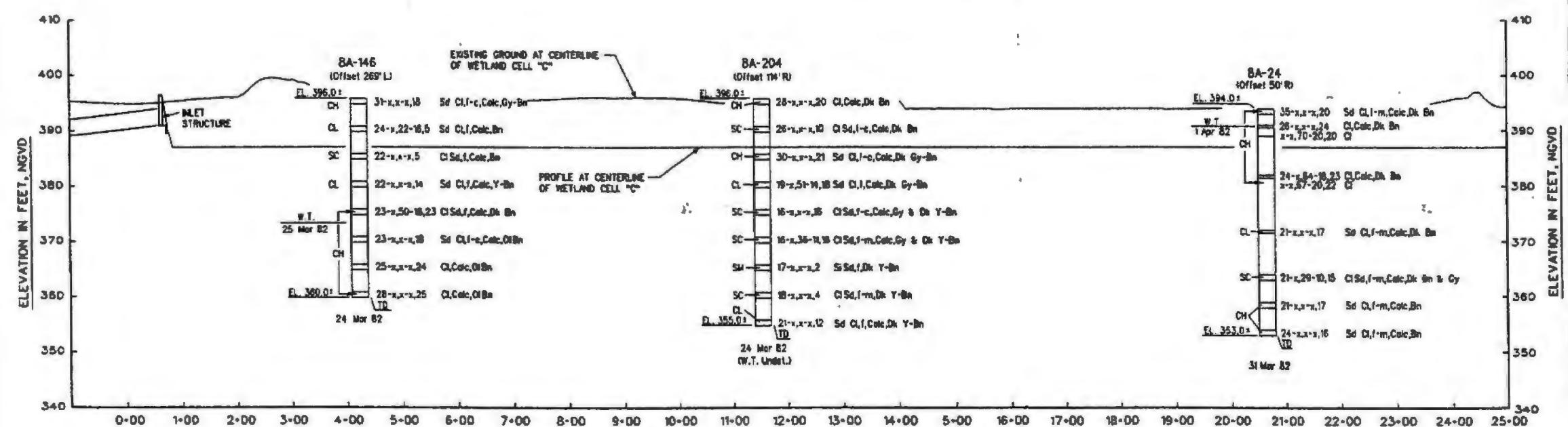
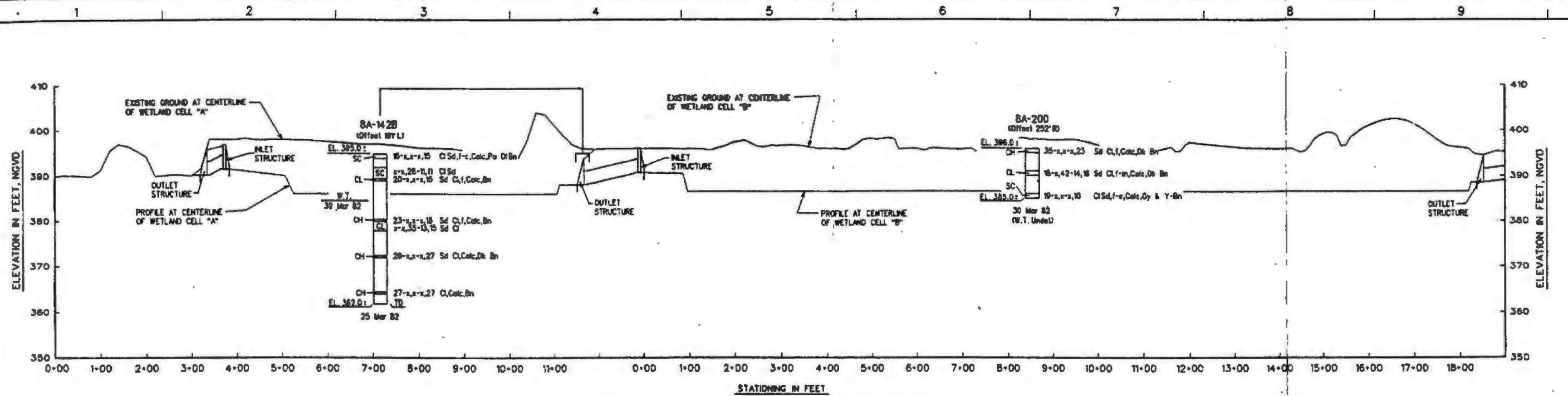
3. USE THESE SHEETS FOR PROJECT "CONCEPT" ONLY. MINOR VARIATIONS EXIST BETWEEN THE PROJECT FEATURES SHOWN AND THE PROJECT LAYOUT AND DESIGN AS CURRENTLY FORMULATED. REFER TO CIVIL/STRUCTURAL DESIGN AND RELOCATIONS APPENDIX FOR CORRECT LAYOUT.



SYNTHOIC NO.	SECTION	DATE	DESCRIPTION OF DRAWING	U.S. ARMY ENGINEER DISTRICT, FORT WORTH
			ENGINEERING DIVISION	DEPT. OF ENGINEERS
			CIVIL PLANNING AND ENGINEERING BRANCH	FORT WORTH, TEXAS
RECORDED BY	DALLAS FLOODWAY EXTENSION			
— J. WILLIAMS —	TRINITY RIVER PROJECT, TEXAS			
DRAWN BY	GENERAL REEVALUATION REPORT			
— J. WILLIAMS —	LOWER CHAIN OF WETLANDS			
REVISED BY	SUBSURFACE PROFILE			
— J. WILLIAMS —	(STATIONS 70-00 THRU 115-39)			
SUBMITTED BY				SOL. NO.
WILLIS L. WALKER, P.E.				DATED
CHIEF, GEOTECHNICAL SECTION				CONTR. NO.
	REFERENCE FILE NAME	REFERENCE FILE DESCRIPTION		SEQUENCE NO.

CADD FILE NAME: CDFE3SPB.DWG

1 2 3 4 5 6 7 8

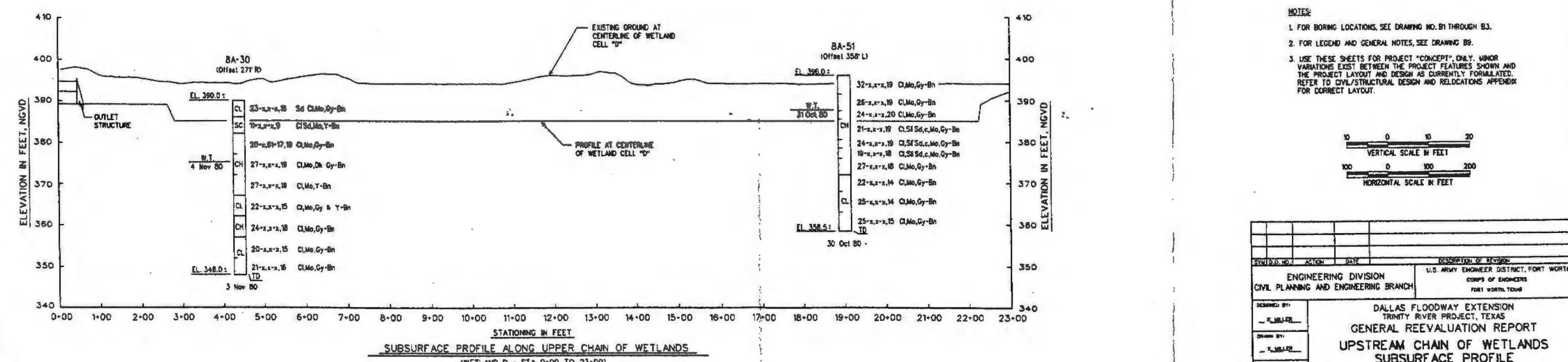
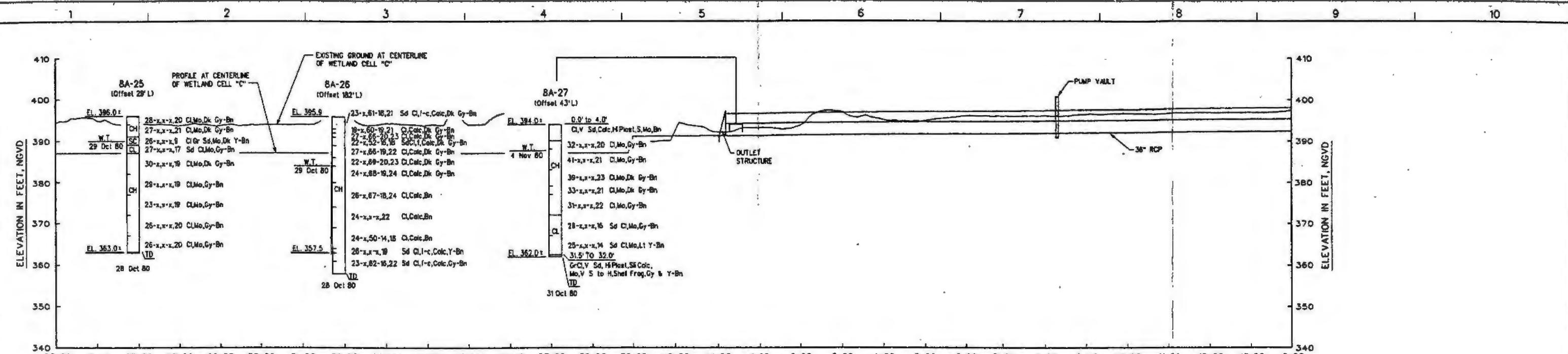


NOTES:

- 1. FOR BORING LOCATIONS, SEE DRAWING NO. B1 THROUGH B3.
- 2. FOR LEGEND AND GENERAL NOTES, SEE DRAWING B8.
- 3. USE THESE SHEETS FOR PROJECT "CONCEPT", ONLY. MINOR VARIATIONS EXIST BETWEEN THE PROJECT FEATURES SHOWN AND THE PROJECT LAYOUT AND DESIGN AS CURRENTLY FORMULATED. REFER TO CIVIL/STRUCTURAL DESIGN AND RELOCATIONS APPENDIX FOR CORRECT LAYOUT.

10 0 10 20
VERTICAL SCALE OF FEET
100 0 100 200

STATION NO.		ACTION	DATE	DESCRIPTION OF DRAWING	
				U.S. ARMY ENGINEER DISTRICT, FORT WORTH CORPS OF ENGINEERS FORT WORTH, TEXAS	
ENGINEERING DIVISION CIVIL PLANNING AND ENGINEERING BRANCH					
DRAWN BY: <u>J. WILLIS</u>	DALLAS FLOODWAY EXTENSION TRINITY RIVER PROJECT, TEXAS				
	GENERAL REEVALUATION REPORT				
	UPPER CHAIN OF WETLANDS SUBSURFACE PROFILE				
DRAWN BY: <u>J. WILLIS</u>	(WETLAND A - STA. 0-00 TO 11-00)				
	(WETLAND B - STA. 0-00 TO 18-00)				
	(WETLAND C - STA. 0-00 TO 25-00)				
REVIEWED BY: <u>J. HALL</u> <u>J. LEBEAU</u>	SOL. NO.	DATED:			
	CONTR. NO.				
	DRAWING NUMBER	SEQUENCE NO.			
	814 OF 16				
SUBMITTED BY: WILLIS L. WALKER, P.E. CHIEF, GEOTECHNICAL SECTION					
CADD FILE NAME: GEEF4SPADON					



NOTES:

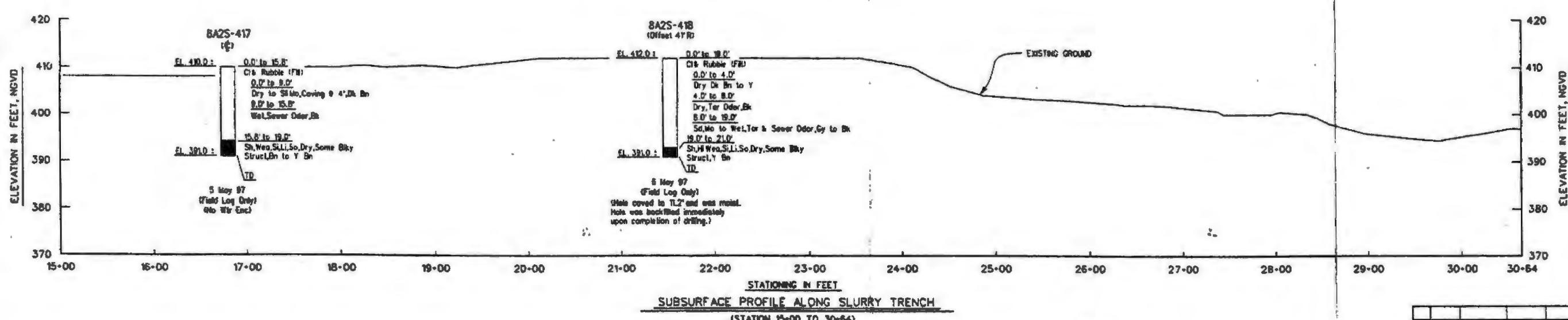
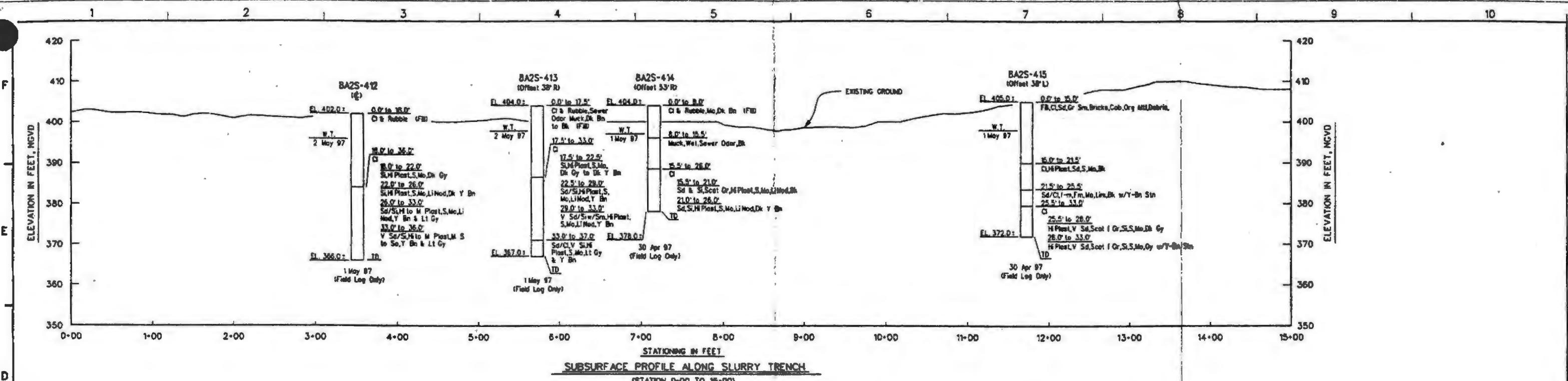
1. FOR BORING LOCATIONS, SEE DRAWING NO. B1 THROUGH B3.
2. FOR LEGEND AND GENERAL NOTES, SEE DRAWING B9.
3. USE THESE SHEETS FOR PROJECT "CONCEPT" ONLY. MINOR VARIATIONS EXIST BETWEEN THE PROJECT FEATURES SHOWN AND THE PROJECT LAYOUT AND DESIGN AS CURRENTLY FORMULATED. REFER TO CIVIL/STRUCTURAL DESIGN AND RELOCATIONS APPENDIX FOR CORRECT LAYOUT.

10 0 10 20
VERTICAL SCALE IN FEET
100 0 100 200
HORIZONTAL SCALE IN FEET

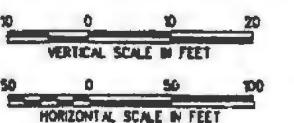
STN. D.O. NO.	ACTION	DATE	DESCRIPTION OF REVISION
			U.S. ARMY ENGINEER DISTRICT, FORT WORTH CORPS OF ENGINEERS FORT WORTH, TEXAS
DESIGNED BY: J. MILLER			DALLAS FLOODWAY EXTENSION TRINITY RIVER PROJECT, TEXAS
DRAWN BY: J. MILLER			GENERAL REEVALUATION REPORT
REVIEWED BY: J. MILLER L. BRONAT			UPSTREAM CHAIN OF WETLANDS SUBSURFACE PROFILE
SUBMITTED BY: WILLIS L. WALKER, P.E. CHEF, GEOTECHNICAL SECTION	SOL. NO.	DATED:	(WETLAND C - STA 25.00 TO 41.00) (WETLAND D - STA 0.00 TO 23.00)
	CONT. NO.		
	SEQUENCE NO.		
	DRAWING NUMBER		
	SHEET NO.		
	NO.		

DOFEYB2.DWG
SHEET BORDER & TITLE BLOCK
REFERENCE FILE NAME
REFERENCE FILE DESCRIPTION

JADDF FILE NAME: GOFEGP.DBDN



- NOTES:**
1. FOR BORING LOCATIONS, SEE DRAWING NO. B1 THROUGH B3.
 2. FOR LEGEND AND GENERAL NOTES, SEE DRAWING B9.
 3. USE THESE SHEETS FOR PROJECT "CONCEPT", ONLY. MINOR VARIATIONS EXIST BETWEEN THE PROJECT FEATURES SHOWN AND THE PROJECT LAYOUT AND DESIGN AS CURRENTLY FORMULATED. REFER TO CIVIL/STRUCTURAL DESIGN AND RELOCATIONS APPENDIX FOR CORRECT LAYOUT.



SECTION	SHEET NUMBER & TITLE BLOCK
WILLIS L. WALKER, P.E. CHIEF, GEOTECHNICAL SECTION	DRAWING NUMBER
	SEQUENCE NO. B16 OF 16

SYNTH. NO.	ACTION	DATE	DESCRIPTION OF DRAWING
			U.S. ARMY ENGINEER DISTRICT, FORT WORTH CORPS OF ENGINEERS FORT WORTH, TEXAS
			GENERAL REEVALUATION REPORT
			DALLAS FLOODWAY EXTENSION TRINITY RIVER PROJECT, TEXAS
			SLURRY TRENCH SUBSURFACE PROFILE (STATION 0-00 TO 30-64)
SEARCHED BY: K. WALKER	INDEXED BY: K. WALKER	REVIEWED BY: J. HALL A. COOPER	SOL. NO. DATED: CONTR. NO. DRAWING NUMBER SHEET NO. SEQUENCE NO. B16 OF 16
SEARCHED BY: INDEXED BY: REVIEWED BY: SOL. NO. DATED: CONTR. NO. DRAWING NUMBER SHEET NO. SEQUENCE NO. B16 OF 16	INDEXED BY: REVIEWED BY: SOL. NO. DATED: CONTR. NO. DRAWING NUMBER SHEET NO. SEQUENCE NO. B16 OF 16	INDEXED BY: REVIEWED BY: SOL. NO. DATED: CONTR. NO. DRAWING NUMBER SHEET NO. SEQUENCE NO. B16 OF 16	INDEXED BY: REVIEWED BY: SOL. NO. DATED: CONTR. NO. DRAWING NUMBER SHEET NO. SEQUENCE NO. B16 OF 16

CADD FILE NAME: GDEPSSAD00