Figure 1

RAILROAD OPENING AND PROPOSED RAILROAD DITCH

WHARTON INTERIM FEASIBILITY STUDY

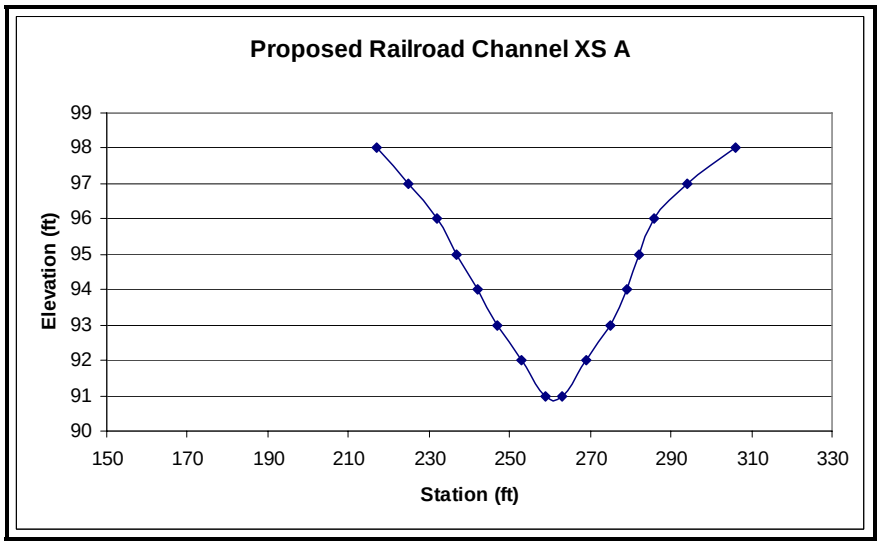
U.S. Army Corps of Engineers

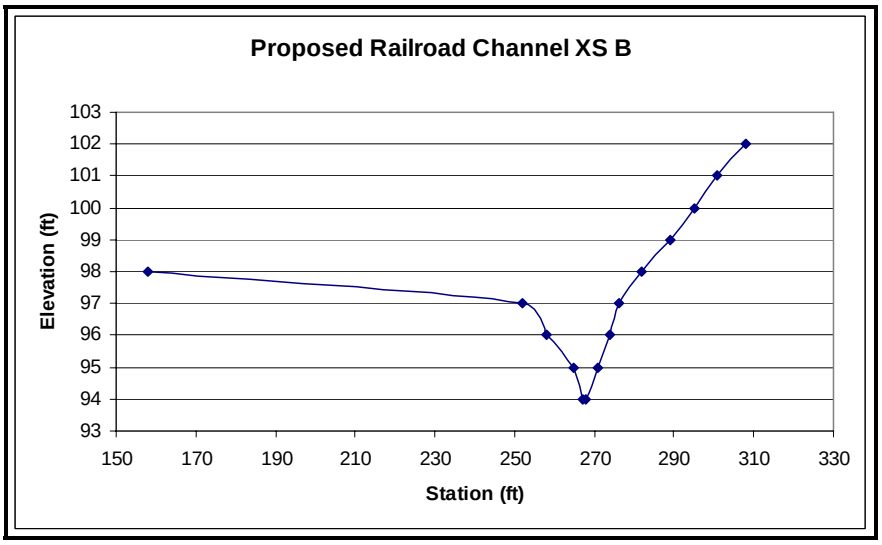
Halff Associates, Inc.

ENGINEERS · ARCHITECTS · SCIENTISTS · PLANNERS · SURVEYORS

DATE: JULY 7, 2006 AVO: 20819

DGN: B1SPROPRR_DITCH.DGN





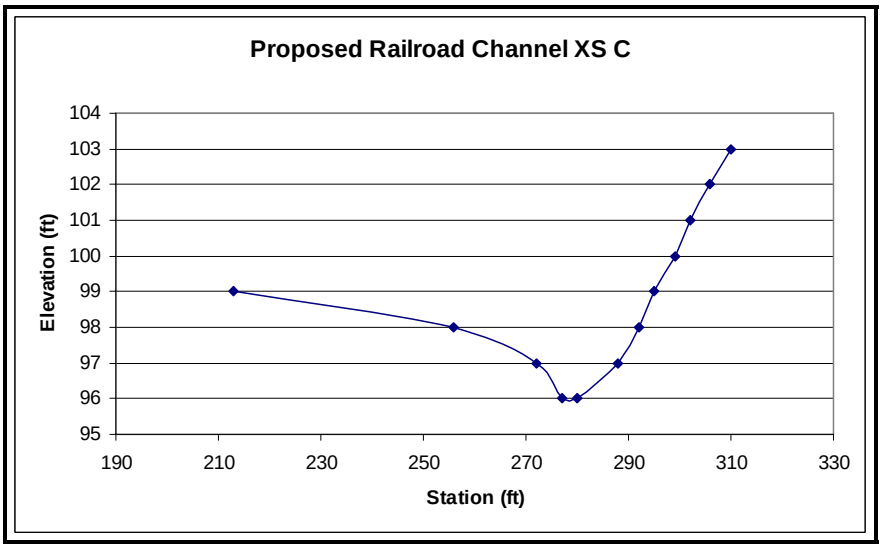


Figure 2. Proposed Railroad Channel Cross-Sections

LEGEND

SUB-BASIN

AREA, ACRES

DRAINAGE AREA DIVIDE

BOSMAN SLOUGH

OTHER CREEK

FM 102

COLORADO RIVER

N

SCALE IN FEET

Figure 3

OVERALL SUMP DRAINAGE AREA

WHARTON INTERIM FEASIBILITY STUDY

U.S. Army Corps of Engineers

Halff Associates, Inc.

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DATE: JULY 7, 2006 AVO: 20819

DGN: 819OVERALL.DGN

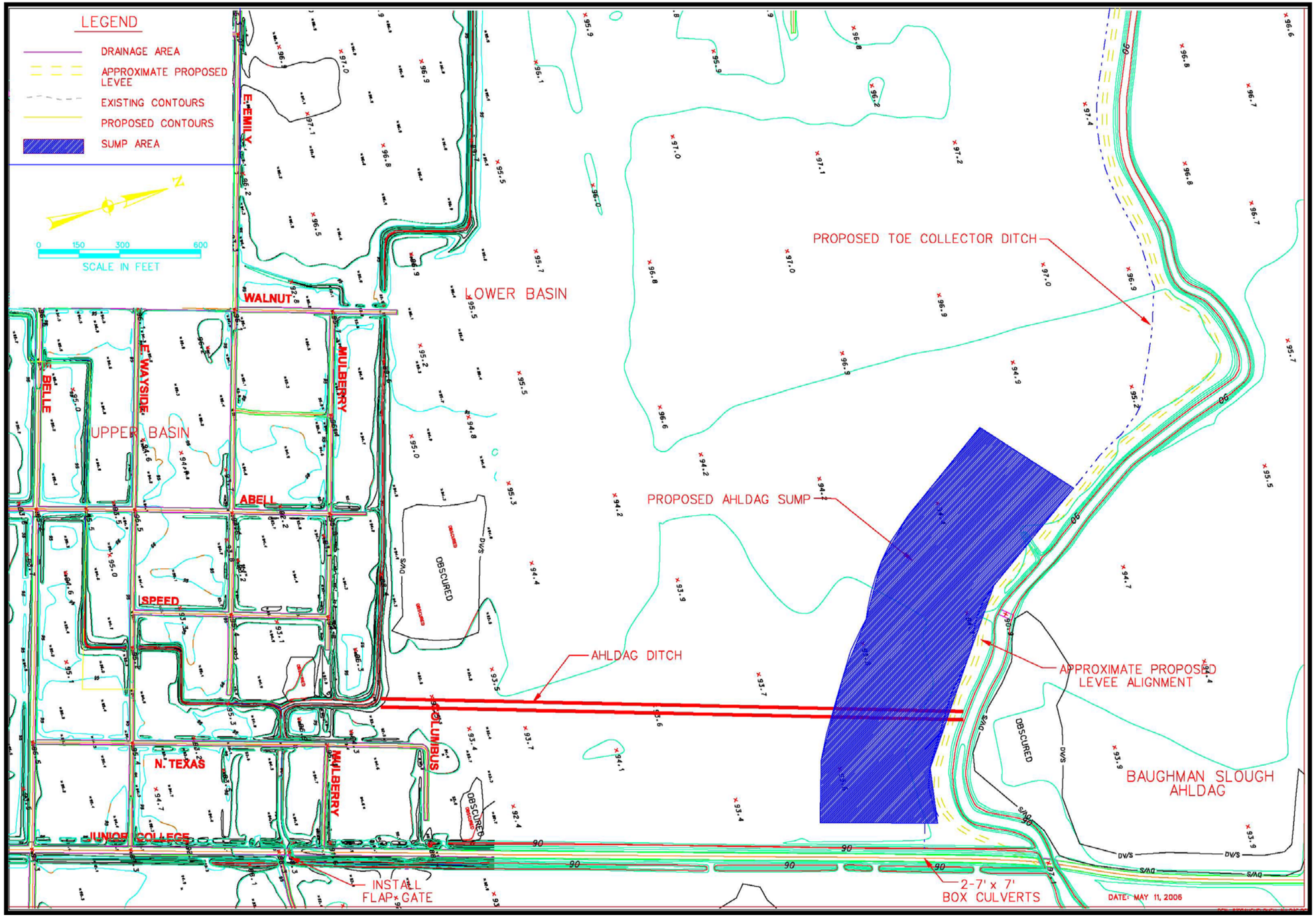


Figure 4. Corps' Proposed Baughman Slough – Ahldag Excavation

▲U/S Face of Bridge Opening 1

▼ D/S Face of Bridge Opening 2

▲U/S Face of Culvert Opening 1 (2-36” Culverts)

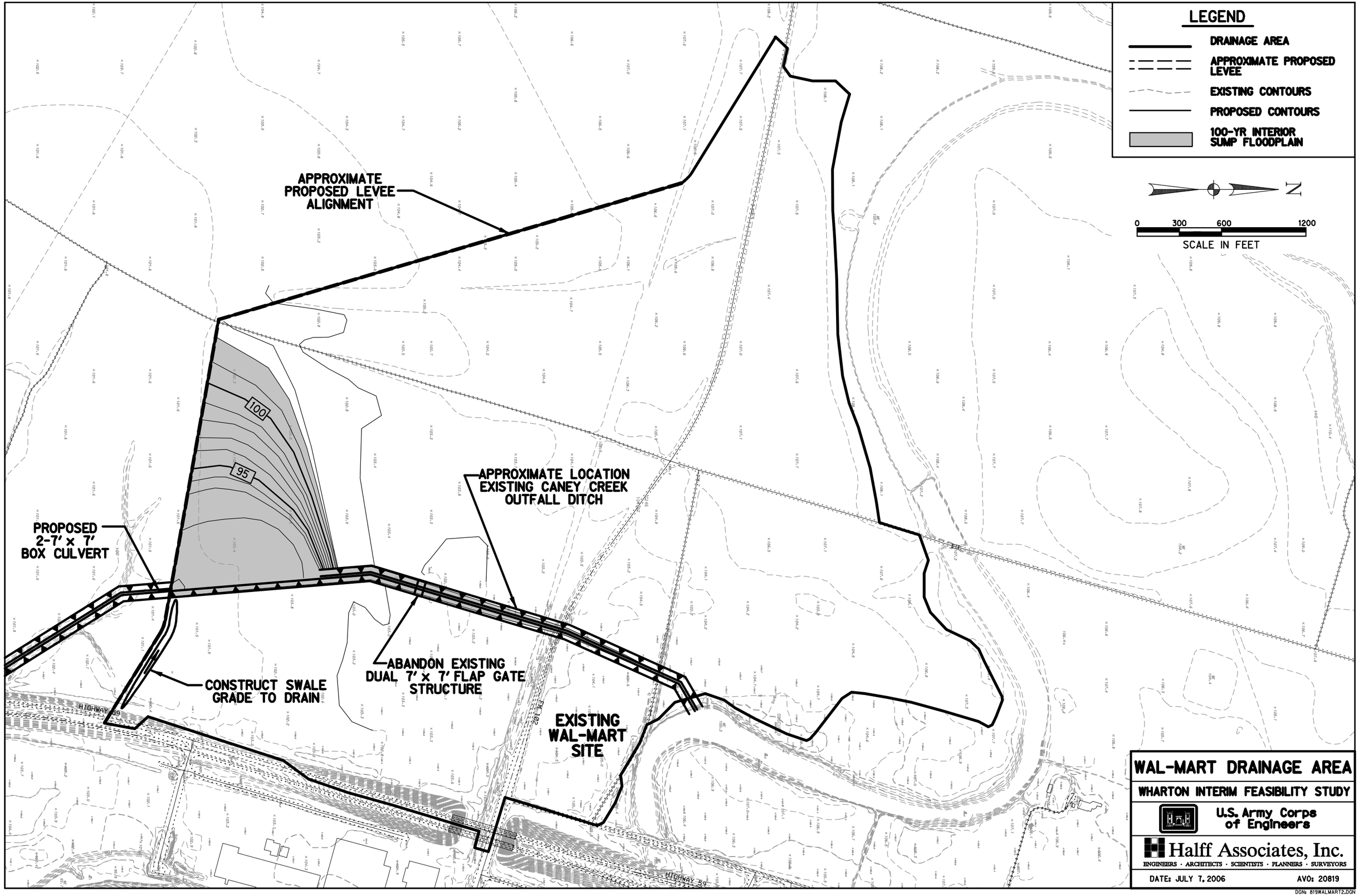
▼U/S Face of Culvert Opening 2 (2-36” Culverts)

▲U/S Channel of Culvert Opening 3 (2-36” Culverts)

Wal-Mart/CC Sump Area	
Hydrology	
Total Area Draining to Sump (acres)	449
Typical Land Use	Pasture/Farm
Composite Curve Number	73.6
25-Year 2-Day Peak Flow (cfs)	1520
25-Year 2-Day Total Inflow Volume (ac-ft)	278
50-Year 2-Day Peak Flow (cfs)	1755
50-Year 2-Day Total Inflow Volume (ac-ft)	327.1
100-Year 2-Day Peak Flow (cfs)	1980
100-Year 2-Day Total Inflow Volume (ac-ft)	383.8
Sluice Details	
Colorado River Tailwater Elevation (ft)	101.26
Sluice Size	2 - 7' x 7' Boxes
Sluice Length (ft)	86
U/S Flowline (ft)	91
D/S Flowline (ft)	90
Peak 25-Year Outflow (cfs)	55
Peak 50-Year Outflow (cfs)	125
Peak 100-Year Outflow (cfs)	235
Sump Details	
25-Year Peak Elevation (ft)	101.4
25-Year Max Storage (ac-ft)	228.3
50-Year Peak Elevation (ft)	101.5
50-Year Max Storage (ac-ft)	236.4
100-Year Peak Elevation (ft)	101.6
100-Year Max Storage (ac-ft)	249.5
# of Properties Affected	1

Additional Comments/Details

Will need to construct swale along levee to direct runoff to sump/sluice.
Will need to abandon existing 2 - 7' x 7' flap gate structure.



WAL-MART DRAINAGE AREA

WHARTON INTERIM FEASIBILITY STUDY

U.S. Army Corps of Engineers

Halff Associates, Inc.

ENGINEERS · ARCHITECTS · SCIENTISTS · PLANNERS · SURVEYORS

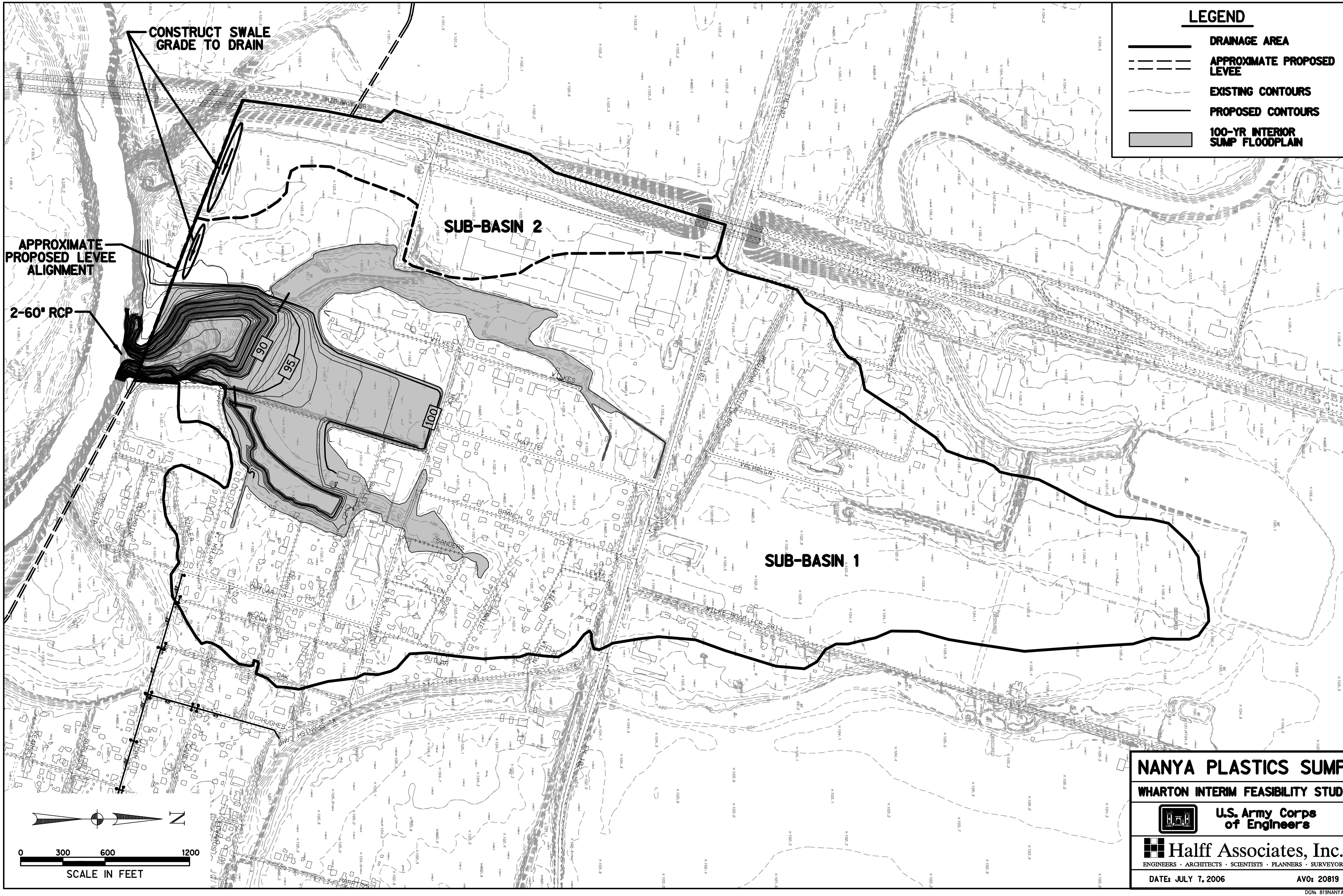
DATE: JULY 7, 2006 AVO: 20819

DGN: 819WALMART2.DGN

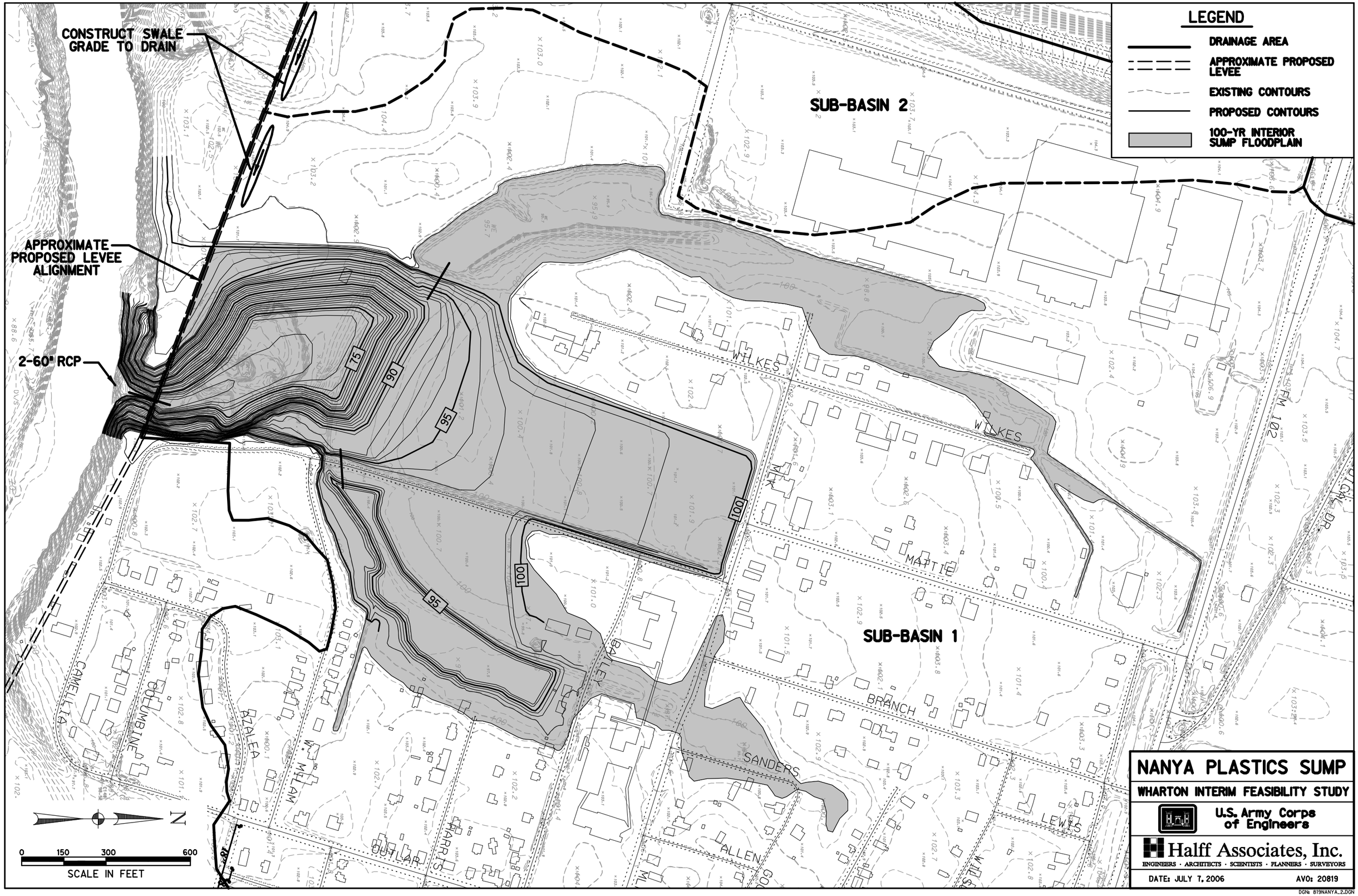
Nanya Plastics Sump Area	
Hydrology	
Sub-Basin 1	
Total Area Draining to Sump (acres)	390
Typical Land Use	Commercial/Residential
Composite Curve Number	83.6
25-Year 2-Day Peak Flow (cfs)	1435
25-Year 2-Day Total Inflow Volume (ac-ft)	284.2
50-Year 2-Day Peak Flow (cfs)	1625
50-Year 2-Day Total Inflow Volume (ac-ft)	328.7
100-Year 2-Day Peak Flow (cfs)	1805
100-Year 2-Day Total Inflow Volume (ac-ft)	379.6
Sub-Basin 2	
Total Area Draining to Sump (acres)	50.5
Typical Land Use	Industrial
Composite Curve Number	87.3
25-Year 2-Day Peak Flow (cfs)	235
25-Year 2-Day Total Inflow Volume (ac-ft)	38.8
50-Year 2-Day Peak Flow (cfs)	265
50-Year 2-Day Total Inflow Volume (ac-ft)	44.6
100-Year 2-Day Peak Flow (cfs)	290
100-Year 2-Day Total Inflow Volume (ac-ft)	51.3
Sluice Details	
Colorado River Tailwater Elevation (ft)	100.47
Sluice Size (in)	2 - 60"
Sluice Length (ft)	200
U/S Flowline (ft)	70.25
D/S Flowline (ft)	70
Peak 25-Year Outflow (cfs)	0
Peak 50-Year Outflow (cfs)	30
Peak 100-Year Outflow (cfs)	85
Sump Details	
25-Year Peak Elevation (ft)	100.2
25-Year Max Storage (ac-ft)	323.1
50-Year Peak Elevation (ft)	100.6
50-Year Max Storage (ac-ft)	344.3
100-Year Peak Elevation (ft)	100.8
100-Year Max Storage (ac-ft)	356.5
# of Properties Affected	21

Additional Comments/Details

Will need to construct swale along levee to direct runoff to sump/sluice.



LEGEND
DRAINAGE AREA
APPROXIMATE PROPOSED LEVEE
EXISTING CONTOURS
PROPOSED CONTOURS
100-YR INTERIOR SUMP FLOODPLAIN
CONSTRUCT SWALE GRADE TO DRAIN
APPROXIMATE PROPOSED LEVEE ALIGNMENT
2-60" RCP
SUB-BASIN 2
SUB-BASIN 1
HIGHWAY 59
FM 102
WILKES
MATTIE
VALHALLA
BRANCH
SANDERS
LEWIS
ALLEN
WILKE ROAD (CR 281)
OAK
HUGHES
90
95
100
N
0 300 600 1200
SCALE IN FEET
NANYA PLASTICS SUMP
WHARTON INTERIM FEASIBILITY STUDY
U.S. Army Corps of Engineers
Halff Associates, Inc.
ENGINEERS · ARCHITECTS · SCIENTISTS · PLANNERS · SURVEYORS
DATE: JULY 7, 2006
AVO: 20819
DGN: 819NANYA.DGN



NANYA PLASTICS SUMP

WHARTON INTERIM FEASIBILITY STUDY

U.S. Army Corps of Engineers

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DATE: JULY 7, 2006 AVO: 20819

DGN: 819NANYA_2.DGN

Hughes Street Sump Area	
Hydrology	
Upper Basin	
Total Area Draining to Sump (acres)	343
Typical Land Use	Open/Commercial
Composite Curve Number	75.2
25-Year 2-Day Peak Flow (cfs)	1135
25-Year 2-Day Total Inflow Volume (ac-ft)	218.9
50-Year 2-Day Peak Flow (cfs)	1305
50-Year 2-Day Total Inflow Volume (ac-ft)	256.7
100-Year 2-Day Peak Flow (cfs)	1475
100-Year 2-Day Total Inflow Volume (ac-ft)	300.4
Lower Basin	
Total Area Draining to Sump (acres)	132
Typical Land Use	Open/Residential
Composite Curve Number	79.3
25-Year 2-Day Peak Flow (cfs)	520
25-Year 2-Day Total Inflow Volume (ac-ft)	90.2
50-Year 2-Day Peak Flow (cfs)	590
50-Year 2-Day Total Inflow Volume (ac-ft)	105
100-Year 2-Day Peak Flow (cfs)	660
100-Year 2-Day Total Inflow Volume (ac-ft)	122.1
Sluice Details	
Colorado River Tailwater Elevation (ft)	99.96
Sluice Size (in)	2 - 7' x 7' Boxes
Sluice Length (ft)	100
U/S Flowline (ft)	80.8
D/S Flowline (ft)	80
Peak 25-Year Outflow (cfs)	0
Peak 50-Year Outflow (cfs)	25
Peak 100-Year Outflow (cfs)	100
Sump Details	
25-Year Peak Elevation (ft)	99.1
25-Year Max Storage (ac-ft)	309
50-Year Peak Elevation (ft)	99.97
50-Year Max Storage (ac-ft)	352
100-Year Peak Elevation (ft)	100
100-Year Max Storage (ac-ft)	353.6
# of Properties Affected	9

Additional Comments/Details

Hughes Street Sump is a combination of proposed sump storage as well as the Caney Creek storage.
The two sump areas will be connected with the proposed 3-60" pipes under Hughes Street.
Will need to plug and abandon existing 48" outfall near the landfill.

APPROXIMATE PROPOSED LEVEE ALIGNMENT

CONSTRUCT SWALE GRADE TO DRAIN

2-7' X 7' BOX CULVERT

PLUG AND ABANDON EXISTING 48" RCP

EXISTING LANDFILL STORM DRAIN AND FLAP GATE

LOWER BASIN

PROPOSED 3-60" PIPES

UPPER BASIN

LEGEND

- DRAINAGE AREA
- APPROXIMATE PROPOSED LEVEE
- EXISTING CONTOURS
- PROPOSED CONTOURS
- 100-YR INTERIOR SUMP FLOODPLAIN

N

0 400 800 1600

SCALE IN FEET

HUGHES STREET SUMP

WHARTON INTERIM FEASIBILITY STUDY

U.S. Army Corps of Engineers

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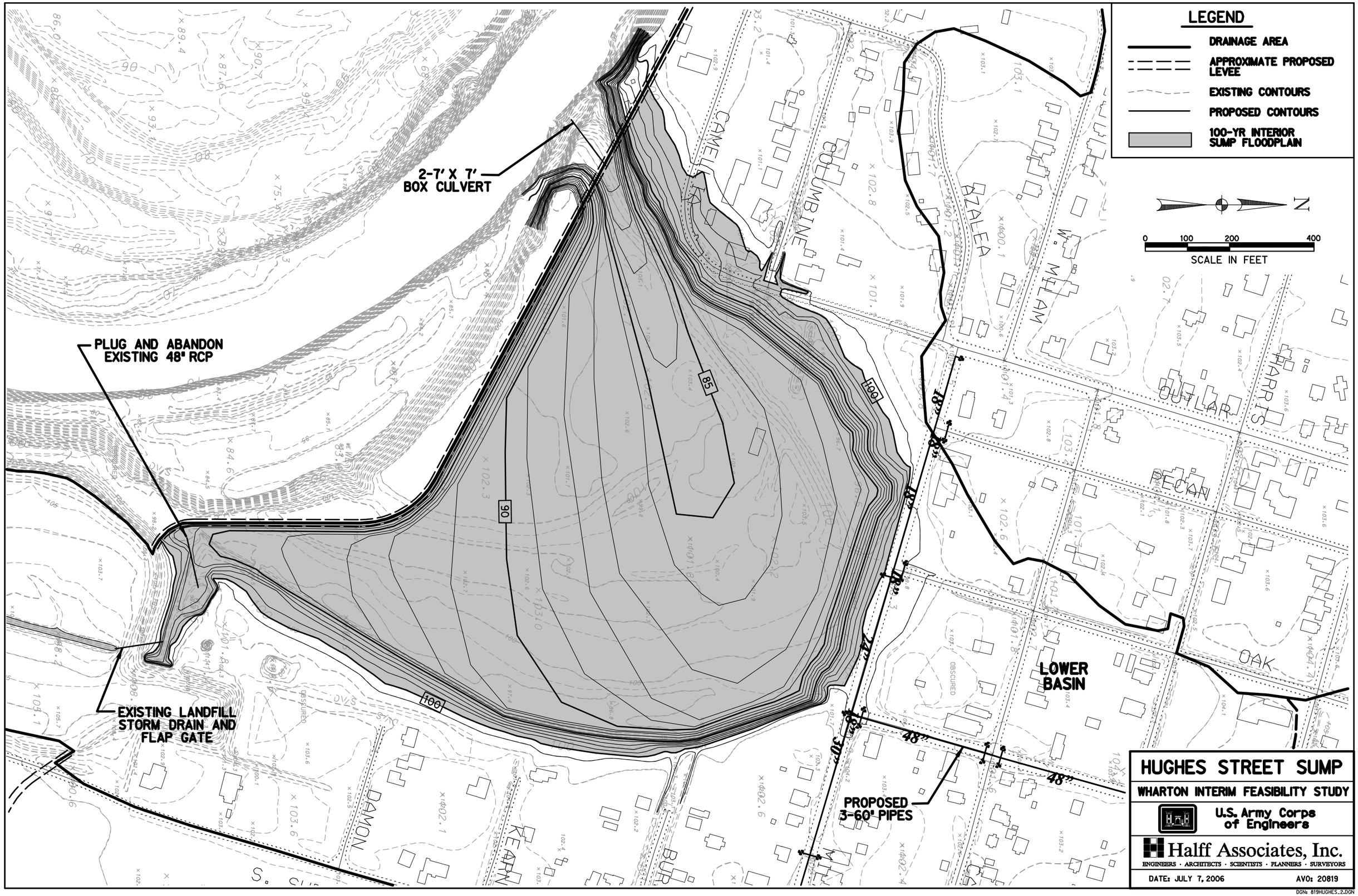
DATE: JULY 7, 2006 AVO: 20819

DGN: 819HUGHES.DGN



HUGHES STREET SUMP

WHARTON INTERIM FEASIBILITY STUDY

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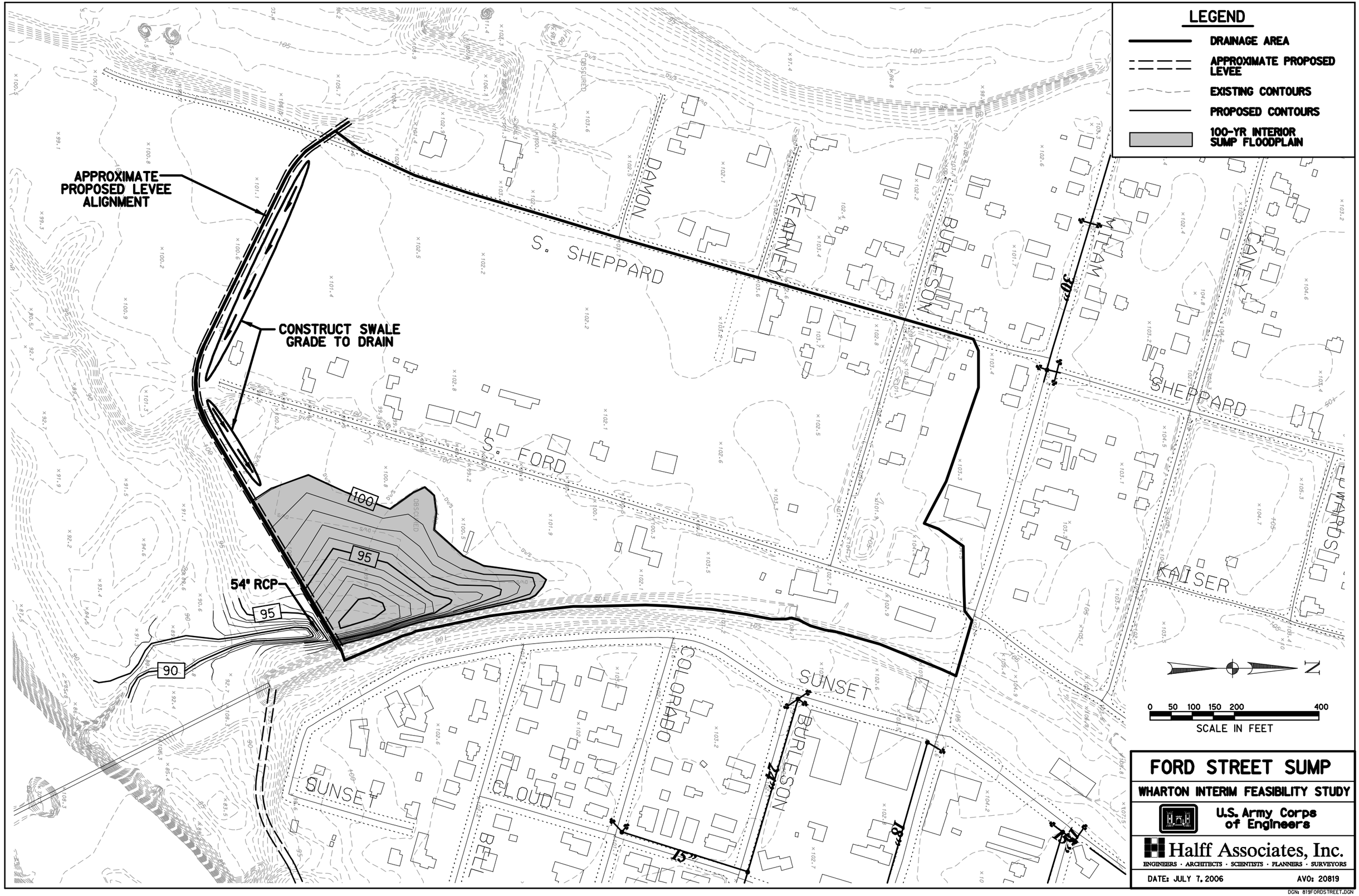
DATE: JULY 7, 2006 AVO: 20819

DGN: 819HUGHES_2.DGN

Ford Street Sump Area	
Hydrology	
Total Area Draining to Sump (acres)	36
Typical Land Use	Residential
Composite Curve Number	81
25-Year 2-Day Peak Flow (cfs)	200
25-Year 2-Day Total Inflow Volume (ac-ft)	25.1
50-Year 2-Day Peak Flow (cfs)	225
50-Year 2-Day Total Inflow Volume (ac-ft)	29.2
100-Year 2-Day Peak Flow (cfs)	250
100-Year 2-Day Total Inflow Volume (ac-ft)	33.9
Sluice Details	
Colorado River Tailwater Elevation (ft)	97.39
Sluice Size (in)	54
Sluice Length (ft)	80
U/S Flowline (ft)	90
D/S Flowline (ft)	89.8
Peak 25-Year Outflow (cfs)	115
Peak 50-Year Outflow (cfs)	125
Peak 100-Year Outflow (cfs)	135
Sump Details	
25-Year Peak Elevation (ft)	99.3
25-Year Max Storage (ac-ft)	7.7
50-Year Peak Elevation (ft)	99.6
50-Year Max Storage (ac-ft)	8.4
100-Year Peak Elevation (ft)	99.9
100-Year Max Storage (ac-ft)	9.2
# of Properties Affected	2

Additional Comments/Details

Will need to construct swale along levee to direct runoff to sump/sluice and intercept borrow ditch flow along Ford Street.



FORD STREET SUMP

WHARTON INTERIM FEASIBILITY STUDY

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DATE: JULY 7, 2006 AVO: 20819

DGN: 819FORDSTREET.DGN

Sunset	
Hydrology	
Total Area Draining to Sump (acres)	21.4
Typical Land Use	Residential
Composite Curve Number	88.5
25-Year 2-Day Peak Flow (cfs)	155
25-Year 2-Day Total Inflow Volume (ac-ft)	16.7
50-Year 2-Day Peak Flow (cfs)	170
50-Year 2-Day Total Inflow Volume (ac-ft)	19.2
100-Year 2-Day Peak Flow (cfs)	190
100-Year 2-Day Total Inflow Volume (ac-ft)	22.0
Sluice Details	
Colorado River Tailwater Elevation (ft)	97.24
Sluice Size (in)	48
Sluice Length (ft)	50
U/S Flowline (ft)	87.5
D/S Flowline (ft)	86.5
Peak 25-Year Outflow (cfs)	5
Peak 50-Year Outflow (cfs)	15
Peak 100-Year Outflow (cfs)	30
Sump Details	
25-Year Peak Elevation (ft)	97.3
25-Year Max Storage (ac-ft)	12.9
50-Year Peak Elevation (ft)	97.4
50-Year Max Storage (ac-ft)	13.1
100-Year Peak Elevation (ft)	97.6
100-Year Max Storage (ac-ft)	13.6
# of Properties Affected	8

Additional Comments/Details

LEGEND

- DRAINAGE AREA
- APPROXIMATE PROPOSED LEVEE
- EXISTING CONTOURS
- PROPOSED CONTOURS
- 100-YR INTERIOR SUMP FLOODPLAIN

48" RCP

APPROXIMATE PROPOSED LEVEE ALIGNMENT

90

95

100

S. FORD

SUNSET

COLORADO

CLOUD

BURLESON

GALLAHER

WHARTON

CANEY

RICHMOND ROAD

POLK

ELM

ID ROAD

15"

36"

30"

24"

12"

42"

N

0 50 100 150 200 400

SCALE IN FEET

SUNSET SUMP

WHARTON INTERIM FEASIBILITY STUDY

U.S. Army Corps of Engineers

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DATE: JULY 7, 2006 AVO: 20819

DGN: 819SUNSETSTREET.DGN

LEGEND

- DRAINAGE AREA
- APPROXIMATE PROPOSED LEVEE
- EXISTING CONTOURS
- PROPOSED CONTOURS
- 100-YR INTERIOR SUMP FLOODPLAIN

42"

42"

42"

42"

48" RCP

APPROXIMATE PROPOSED LEVEE ALIGNMENT

SUNSET

N

0 25 50 75 100 200

SCALE IN FEET

SUNSET SUMP

WHARTON INTERIM FEASIBILITY STUDY

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DATE: JULY 7, 2006 AVO: 20819

DGN: 819SUNSETSTREET_2.DGN

Black/Rusk Street Sump Area	
Hydrology	
Black Sub-Basin	
Total Area Draining to Sump (acres)	24
Typical Land Use	Residential/Industrial
Composite Curve Number	88
25-Year 2-Day Peak Flow (cfs)	160
25-Year 2-Day Total Inflow Volume (ac-ft)	18.6
50-Year 2-Day Peak Flow (cfs)	170
50-Year 2-Day Total Inflow Volume (ac-ft)	21.4
100-Year 2-Day Peak Flow (cfs)	195
100-Year 2-Day Total Inflow Volume (ac-ft)	24.5
Rusk Sub-Basin	
Total Area Draining to Sump (acres)	58
Typical Land Use	Open/Commercial
Composite Curve Number	80.5
25-Year 2-Day Peak Flow (cfs)	350
25-Year 2-Day Total Inflow Volume (ac-ft)	40.4
50-Year 2-Day Peak Flow (cfs)	390
50-Year 2-Day Total Inflow Volume (ac-ft)	47
100-Year 2-Day Peak Flow (cfs)	435
100-Year 2-Day Total Inflow Volume (ac-ft)	54.6
Sluice Details	
Colorado River Tailwater Elevation (ft)	96.56
Sluice Size (in)	66
Sluice Length (ft)	110
U/S Flowline (ft)	88
D/S Flowline (ft)	87
Peak 25-Year Outflow (cfs)	265
Peak 50-Year Outflow (cfs)	285
Peak 100-Year Outflow (cfs)	300
Sump Details	
25-Year Peak Elevation (ft)	99.8
25-Year Max Storage (ac-ft)	17.3
50-Year Peak Elevation (ft)	100.2
50-Year Max Storage (ac-ft)	19.1
100-Year Peak Elevation (ft)	100.7
100-Year Max Storage (ac-ft)	20.9
# of Properties Affected	11

Additional Comments/Details

Will need to plug and abandon borrow ditch storm drain pipe.
Will need to construct 3 swales.
Will need to construct curb openings in Elm Street.

LEGEND

- DRAINAGE AREA
- APPROXIMATE PROPOSED LEVEE
- EXISTING CONTOURS
- PROPOSED CONTOURS
- 100-YR INTERIOR SUMP FLOODPLAIN

CONSTRUCT SWALE GRADE TO DRAIN

INSTALL FLAP GATE ON RUSK STREET OUTFALL

CONSTRUCT SWALE GRADE TO DRAIN

APPROXIMATE PROPOSED LEVEE ALIGNMENT

APPROXIMATE LOCATION EXISTING BORROW DITCH STORM DRAIN PLUG AND ABANDON

66" RCP

RICHMOND ROAD

POLK

ELM

FULTON

HOUSTON

BURLESON

HAWES

RUSK

BLACK

MILAM

RESIDENT

DENNIS

WALNUT

FM 1299

S. EAST AVE.

MURPHY

LINS

(RIVER ROAD)

MAPLE

CAROLYN

WASHINGTON

AVE.

HIGHWAY 60

N

0 150 300 600

SCALE IN FEET

BLACK/RUSK STREET SUMP

WHARTON INTERIM FEASIBILITY STUDY

U.S. Army Corps of Engineers

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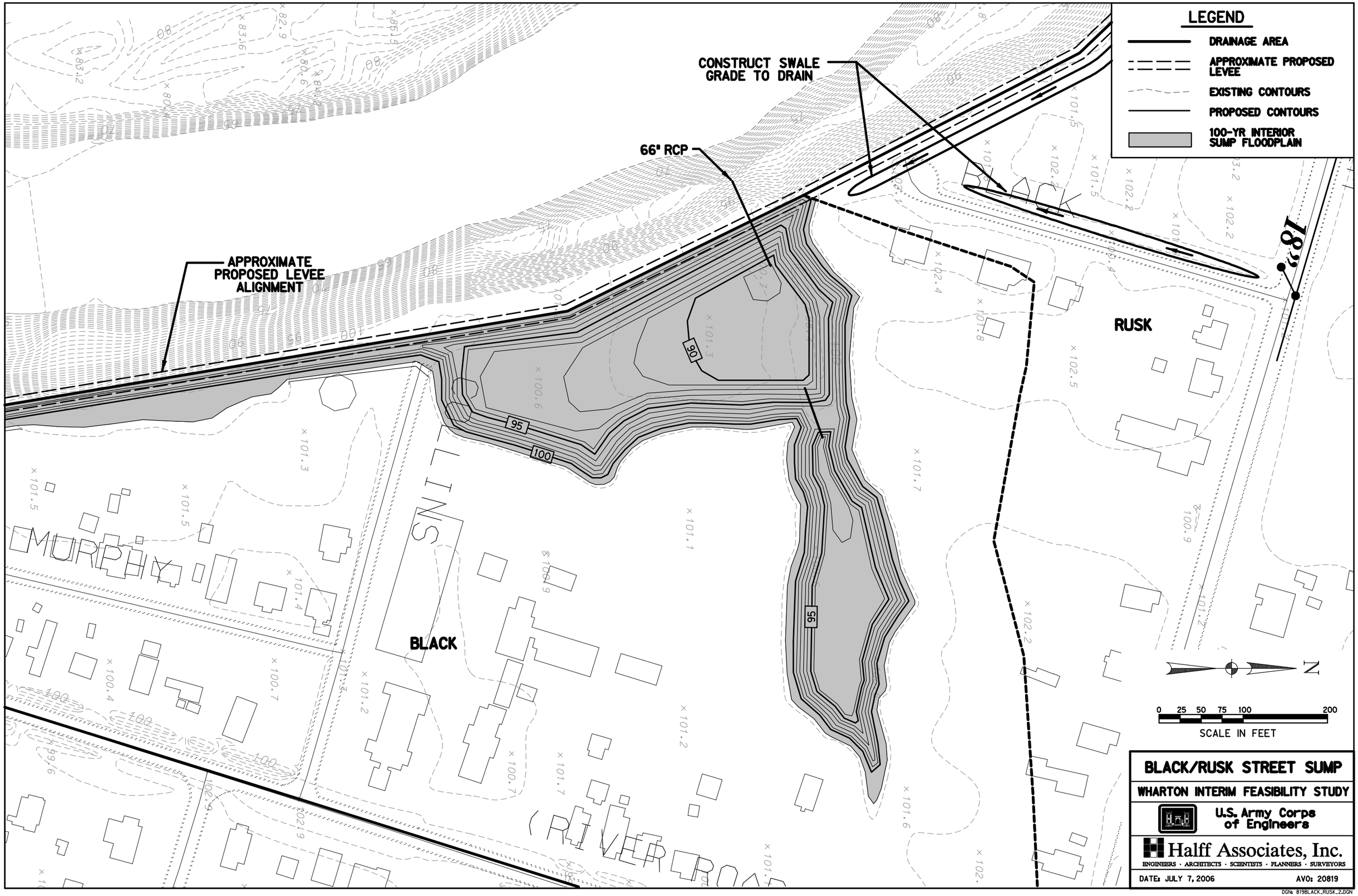
DATE: JULY 7, 2006 AVO: 20819

DGN: 819BLACK_RUSK.DGN



BLACK/RUSK STREET SUMP

WHARTON INTERIM FEASIBILITY STUDY

U.S. Army Corps of Engineers

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DATE: JULY 7, 2006 AVO: 20819

DGN: 819BLACK_RUSK_2.DGN

Alabama Road Sump Area	
Hydrology	
Total Area Draining to Sump (acres)	345
Typical Land Use	Residential
Composite Curve Number	83.3
25-Year 2-Day Peak Flow (cfs)	1710
25-Year 2-Day Total Inflow Volume (ac-ft)	250.5
50-Year 2-Day Peak Flow (cfs)	1920
50-Year 2-Day Total Inflow Volume (ac-ft)	289.8
100-Year 2-Day Peak Flow (cfs)	2120
100-Year 2-Day Total Inflow Volume (ac-ft)	334.9
Sluice Details	
Colorado River Tailwater Elevation (ft)	95.94
Sluice Size	2 - 7' x 7' Boxes
Sluice Length (ft)	260
U/S Flowline (ft)	68.5
D/S Flowline (ft)	63.5
Peak 25-Year Outflow (cfs)	200
Peak 50-Year Outflow (cfs)	450
Peak 100-Year Outflow (cfs)	665
Sump Details	
25-Year Peak Elevation (ft)	96.2
25-Year Max Storage (ac-ft)	163.7
50-Year Peak Elevation (ft)	96.9
50-Year Max Storage (ac-ft)	172.2
100-Year Peak Elevation (ft)	97.8
100-Year Max Storage (ac-ft)	185.1
# of Properties Affected	1

Additional Comments/Details

Will need to construct swale along levee to direct runoff to sump/sluice.

LEGEND

- DRAINAGE AREA
- APPROXIMATE PROPOSED LEVEE
- EXISTING CONTOURS
- PROPOSED CONTOURS
- 100-YR INTERIOR SUMP FLOODPLAIN

CONSTRUCT SWALE GRADE TO DRAIN

2 - 7' x 7' Boxes

CONSTRUCT SWALE GRADE TO DRAIN

APPROXIMATE PROPOSED LEVEE ALIGNMENT

FM 1299

S. EAST AVE.

(RIVER ROAD) (W)

S. ALABAMA

12'X 3' BOX

SARA ANN

JUNRAVEN

SALISBURY

ALABAMA

S. EAST AVE.

MAHAN

KINKAID

HOLLIS

N

0 125 250 375 500 1000

SCALE IN FEET

ALABAMA ROAD SUMP

WHARTON INTERIM FEASIBILITY STUDY

U.S. Army Corps of Engineers

Halff Associates, Inc.

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DATE: JULY 7, 2006 AVO: 20819

DGN: 819ALABAMAROAD.DGN

Baughman Slough RR Sump	
Hydrology	
Total Area Draining to Sump (acres)	183.8
Typical Land Use	Residential/Open Space
Composite Curve Number	84.9
25-Year 2-Day Peak Flow (cfs)	665
25-Year 2-Day Total Inflow Volume (ac-ft)	136.7
50-Year 2-Day Peak Flow (cfs)	755
50-Year 2-Day Total Inflow Volume (ac-ft)	157.7
100-Year 2-Day Peak Flow (cfs)	835
100-Year 2-Day Total Inflow Volume (ac-ft)	181.9
Sluice Details	
Baughman Slough Tailwater Elevation (ft)-25YR	99.68
Baughman Slough Tailwater Elevation (ft)-50YR	99.82
Baughman Slough Tailwater Elevation (ft)-100YR	99.94
Sluice Size (in)	66
Sluice Length (ft)	20
U/S Flowline (ft)	90.7
D/S Flowline (ft)	90.6
Peak 25-Year Outflow (cfs)	0
Peak 50-Year Outflow (cfs)	0
Peak 100-Year Outflow (cfs)	0
Sump Details	
25-Year Peak Elevation (ft)	95.8
25-Year Max Storage (ac-ft)	136.7
50-Year Peak Elevation (ft)	96.3
50-Year Max Storage (ac-ft)	157.7
100-Year Peak Elevation (ft)	97.0
100-Year Max Storage (ac-ft)	181.9
# of Properties Affected	1

Additional Comments/Details

BAUGHMAN SLOUGH RR SUMP

WHARTON INTERIM FEASIBILITY STUDY

U.S. Army Corps of Engineers

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DATE: JULY 7, 2006 AVO: 20819

DGN: 819BAUGHSLOUGH_RRI.DGN

LEGEND

- DRAINAGE AREA
- APPROXIMATE PROPOSED LEVEE
- EXISTING CONTOURS
- PROPOSED CONTOURS
- 100-YR INTERIOR SUMP FLOODPLAIN

66" RCP

APPROXIMATE PROPOSED LEVEE ALIGNMENT

95

92

91

100

102

100.2

RICHMOND ROAD

N

0 50 100 150 200 400

SCALE IN FEET

BAUGHMAN SLOUGH RR SUMP

WHARTON INTERIM FEASIBILITY STUDY

U.S. Army Corps of Engineers

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ENGINEERS · ARCHITECTS · SCIENTISTS · PLANNERS · SURVEYORS

DATE: JULY 7, 2006 AVO: 20819

DGN: 819BAUGHSLOUGH_RR2.DGN

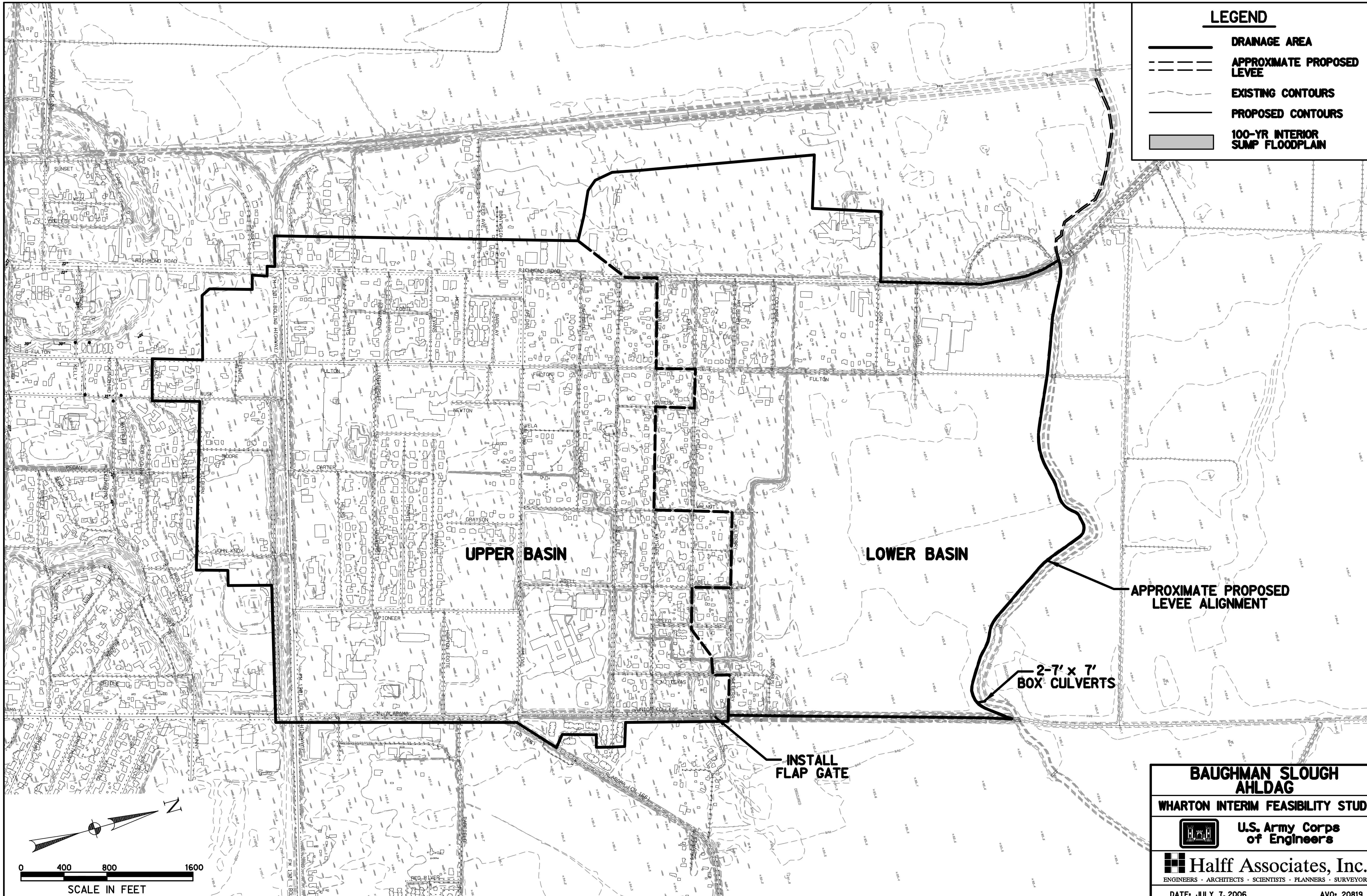
Baughman Slough-Ahldag	
Hydrology	
Lower Basin	
Total Area Draining to Sump (acres)	361.1
Typical Land Use	Residential/Open Space
Composite Curve Number	86
25-Year 2-Day Peak Flow (cfs)	1,055
25-Year 2-Day Total Inflow Volume (ac-ft)	272.6
50-Year 2-Day Peak Flow (cfs)	1,195
50-Year 2-Day Total Inflow Volume (ac-ft)	314.0
100-Year 2-Day Peak Flow (cfs)	1,330
100-Year 2-Day Total Inflow Volume (ac-ft)	361.5
Upper Basin	
Total Area Draining to Sump (acres)	444.3
Typical Land Use	Residential
Composite Curve Number	88.5
25-Year 2-Day Peak Flow (cfs)	1215
25-Year 2-Day Total Inflow Volume (ac-ft)	347.1
50-Year 2-Day Peak Flow (cfs)	1,375
50-Year 2-Day Total Inflow Volume (ac-ft)	398.3
100-Year 2-Day Peak Flow (cfs)	1,535
100-Year 2-Day Total Inflow Volume (ac-ft)	457.0

Sluice Details	**Levee**	**Alabama Rd**
	Sluice 1	**Sluice 2**
Baughman Slough Tailwater Elev. (ft)-25YR	94.09	93.90
Baughman Slough Tailwater Elev. (ft)-50YR	94.25	94.10
Baughman Slough Tailwater Elev. (ft)-100YR	94.39	94.20
Sluice Size	2 - 7' x 7' Boxes	2 - 7' x 7' Boxes
Sluice Length (ft)	80	40
U/S Flowline (ft)	84.0	88.5
D/S Flowline (ft)	83.5	88.0

Sump Details					
	Pond 1	**Pond 2**	**Pond 3**	**Pond 4**	**No Pond**
Peak 25-Year Outflow (cfs)	0	250	465	800	1,230
Peak 50-Year Outflow (cfs)	0	475	725	1,045	1,285
Peak 100-Year Outflow (cfs)	0	755	985	1,125	1,370
25-Year Peak Elevation (ft)	92.2	94.1	94.3	94.7	95.3
25-Year Max Storage (ac-ft)	619.7	461.4	411.1	354.5	171.9
50-Year Peak Elevation (ft)	93.0	94.5	94.7	95.0	95.5
50-Year Max Storage (ac-ft)	712.4	492.8	444.2	388.6	214.8
100-Year Peak Elevation (ft)	93.9	94.8	95.1	95.3	95.8
100-Year Max Storage (ac-ft)	818.6	524.0	479.6	436.0	258.5
# of Properties Affected	6	6	1	1	0

Additional Comments/Details

Install flap gate on culvert under Alabama Road


LEGEND
DRAINAGE AREA
APPROXIMATE PROPOSED LEVEE
EXISTING CONTOURS
PROPOSED CONTOURS
100-YR INTERIOR SUMP FLOODPLAIN
UPPER BASIN
LOWER BASIN
APPROXIMATE PROPOSED LEVEE ALIGNMENT
2-7' x 7' BOX CULVERTS
INSTALL FLAP GATE
RICHMOND ROAD
FULTON
N. ALABAMA
0 400 800 1600
SCALE IN FEET
N
BAUGHMAN SLOUGH AHLDAG
WHARTON INTERIM FEASIBILITY STUDY
U.S. Army Corps of Engineers
Halff Associates, Inc.
ENGINEERS · ARCHITECTS · SCIENTISTS · PLANNERS · SURVEYORS
DATE: JULY 7, 2006
AVO: 20819

LEGEND

- DRAINAGE AREA
- APPROXIMATE PROPOSED LEVEE
- EXISTING CONTOURS
- PROPOSED CONTOURS
- 100-YR INTERIOR SUMP FLOODPLAIN

N

0 100 200 300 400 800

SCALE IN FEET

FULTON

N. RUSK

E. EMILY

WALNUT

LOWER BASIN

E. WAYSIDE

E. BELLE

MULBERRY

UPPER BASIN

ABELL

ABELL

SPEED

AHLDAG

N. TEXAS

JUNIOR COLLEGE

COLUMBUS

MULBERRY

(CR 166)

ALABAMA

90

87

86

85

84

APPROXIMATE PROPOSED LEVEE ALIGNMENT

2-7' x 7' BOX CULVERTS

INSTALL FLAP GATE

BAUGHMAN SLOUGH AHLDAG - POND 1

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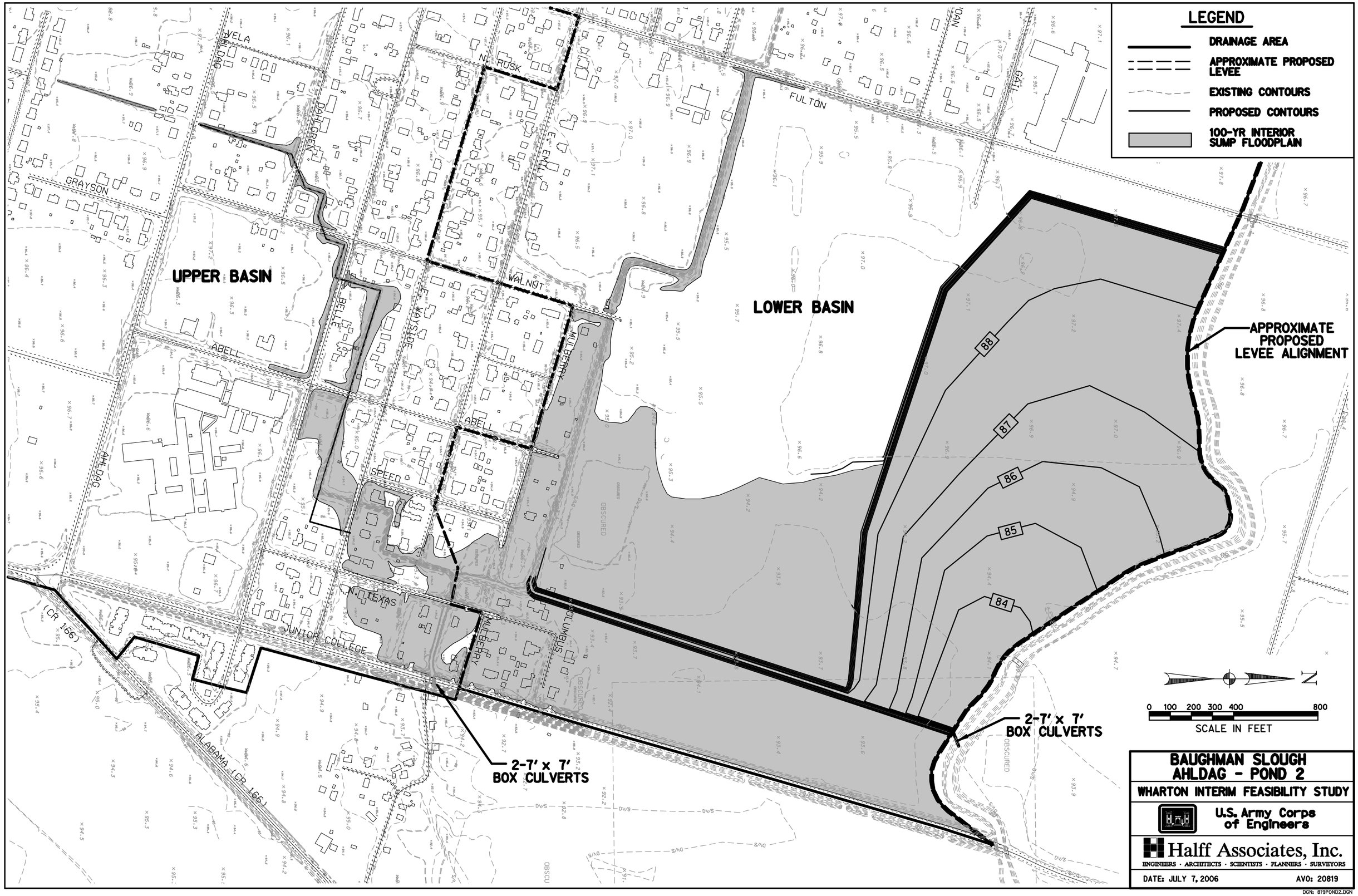

BAUGHMAN SLOUGH
AHLDAG - POND 2
WHARTON INTERIM FEASIBILITY STUDY

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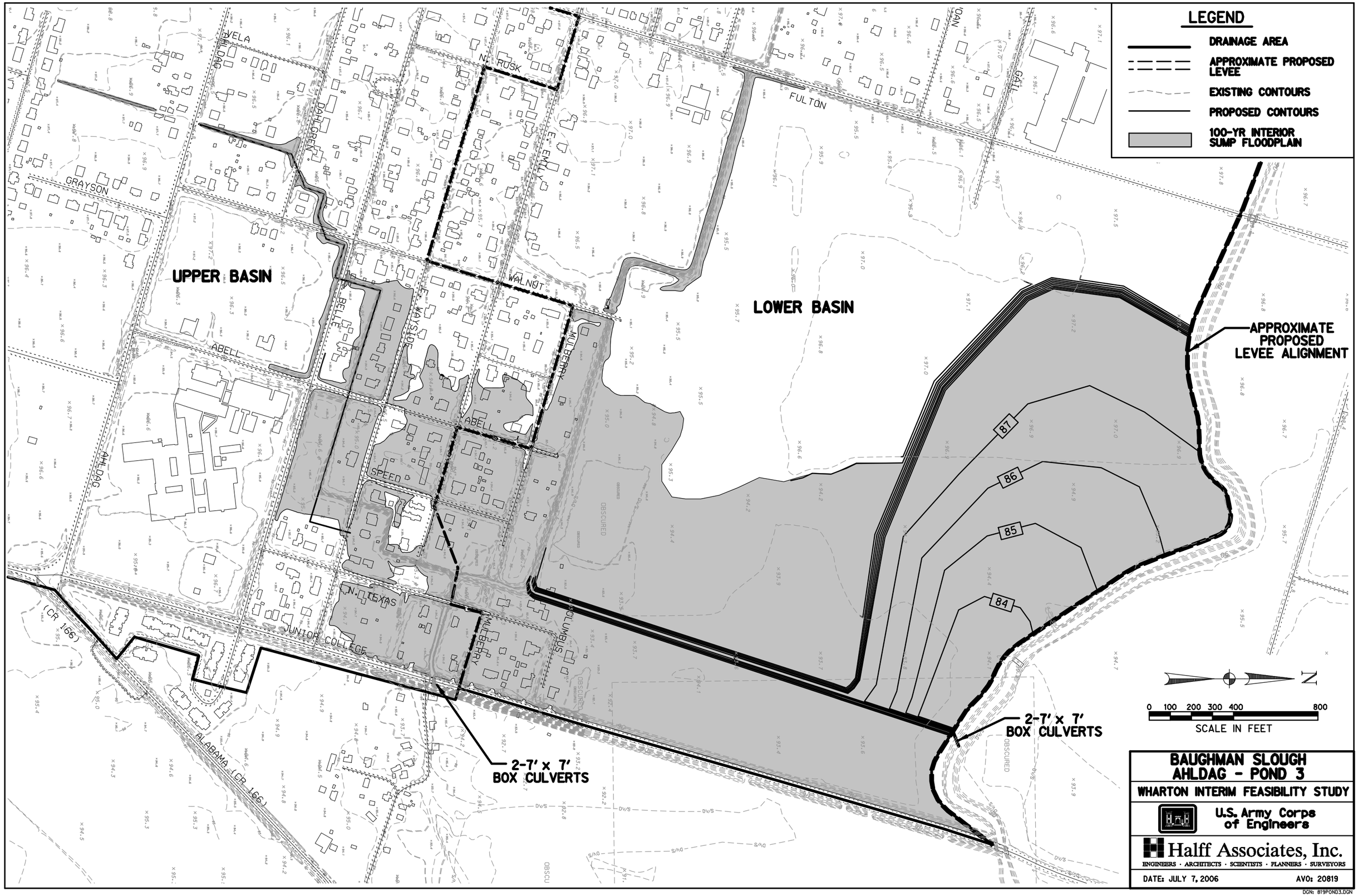
LEGEND
DRAINAGE AREA
APPROXIMATE PROPOSED LEVEE
EXISTING CONTOURS
PROPOSED CONTOURS
100-YR INTERIOR SUMP FLOODPLAIN
UPPER BASIN
LOWER BASIN
APPROXIMATE PROPOSED LEVEE ALIGNMENT
2-7' x 7' BOX CULVERTS
2-7' x 7' BOX CULVERTS
FULTON
RUSK
WALNUT
ABELL
SPEED
GRAYSON
AVELA
AHLDAG
EMILY
WAYSIDE
BELLE
MULBERRY
COLUMBUS
N. TEXAS
JUNIOR COLLEGE
ALABAMA (CR 166)
(CR 166)
87
86
85
84
0 100 200 300 400 800
SCALE IN FEET
BAUGHMAN SLOUGH
AHLDAG - POND 3
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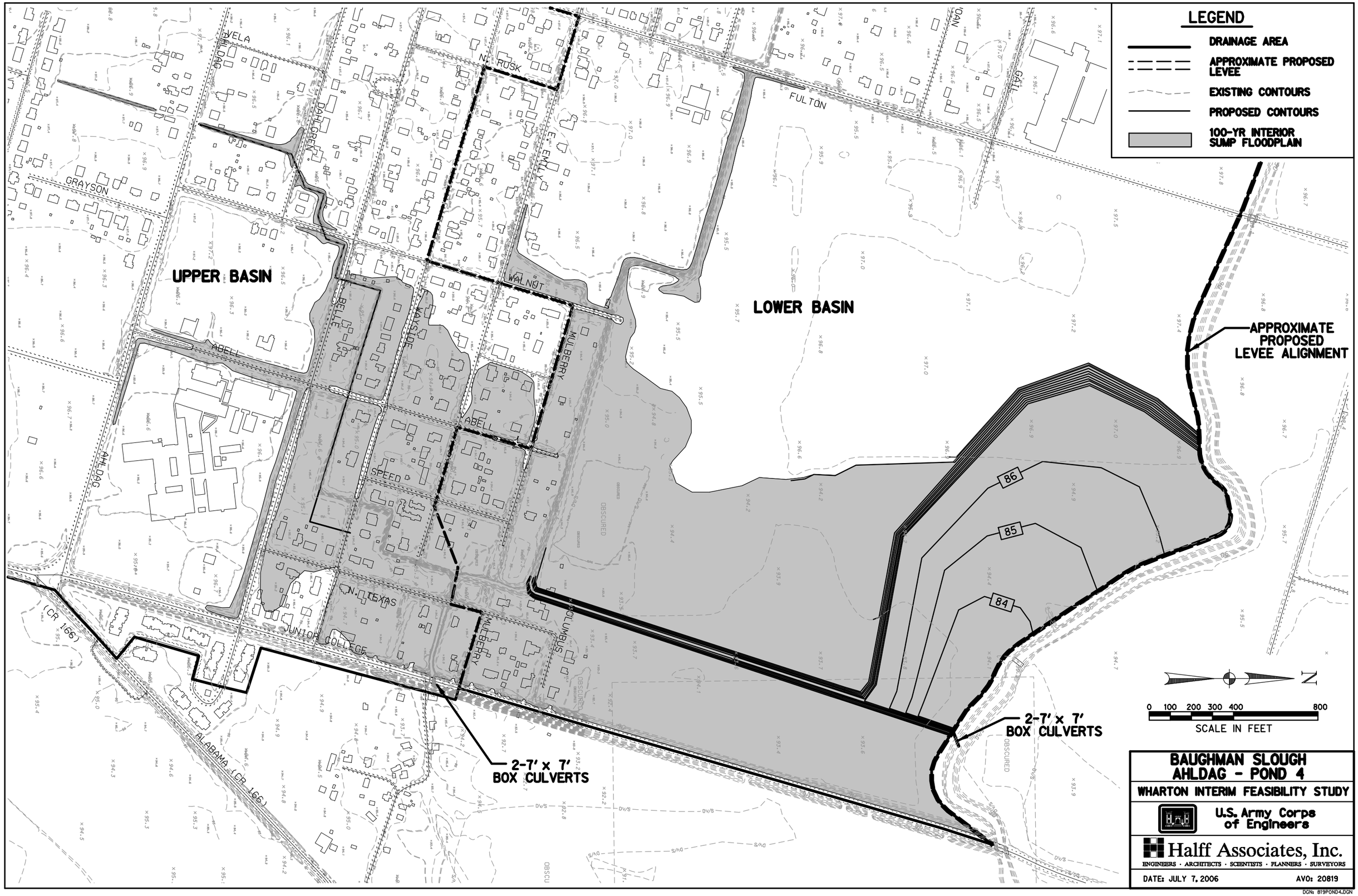

BAUGHMAN SLOUGH AHLDAG - POND 4

WHARTON INTERIM FEASIBILITY STUDY

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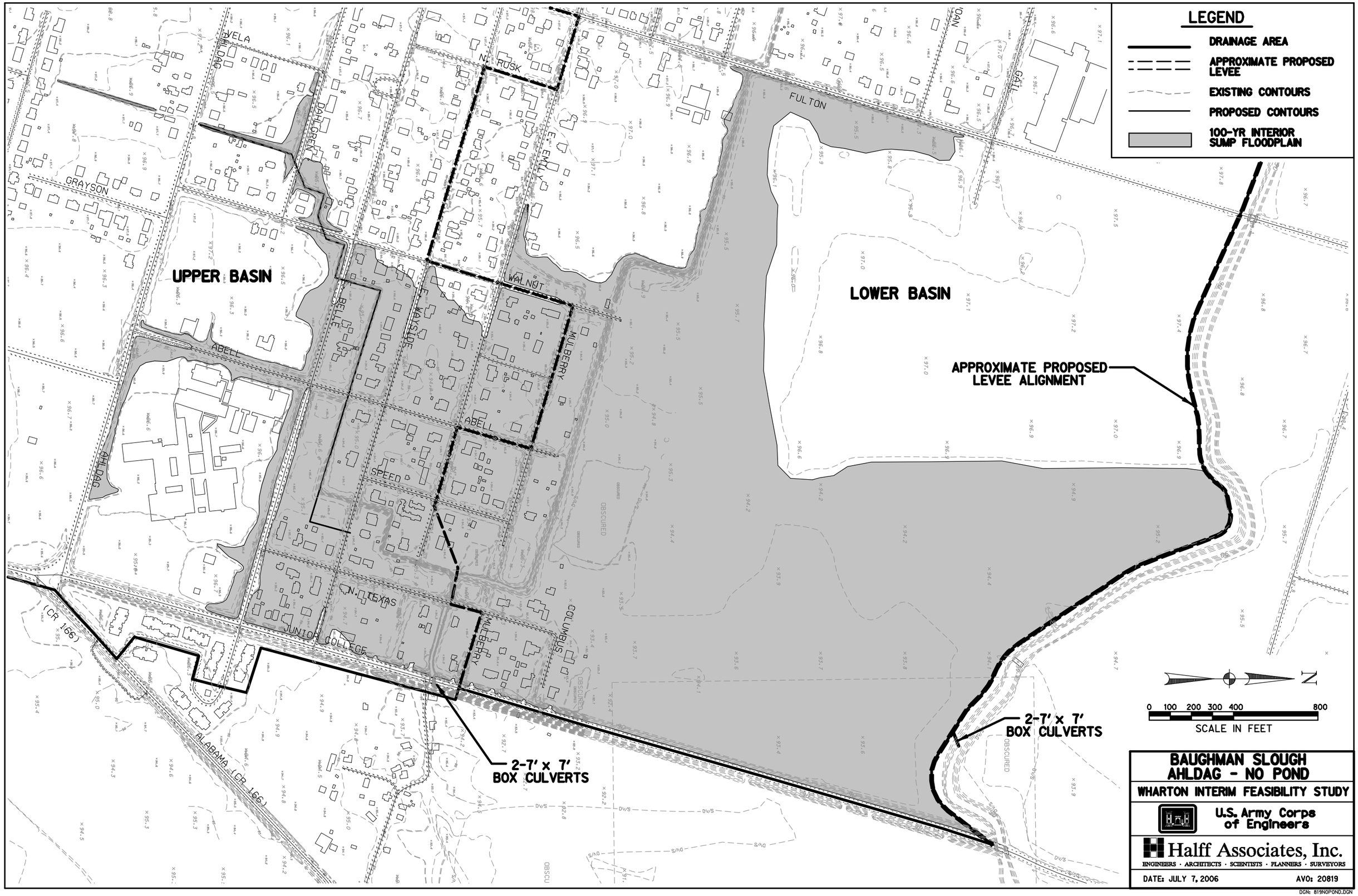

BAUGHMAN SLOUGH
AHLDAG - NO POND

WHARTON INTERIM FEASIBILITY STUDY

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DGN: 819NOPOND.DGN